

KINETICOR HOLDINGS #3 GP LTD.

KINETICOR ASP

MASTER DRAINAGE PLAN



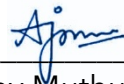
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MASTER DRAINAGE PLAN

Prepared by: 
Ajay Muthukumar, M.Sc., E.I.T.
APEGA ID Number: 299516

Verified by: _____
JF Chenier, P. Eng.
Engineering License Number: APEGA 148679

Corporate Review by: _____
Darryl Reinhardt, P.L. (Eng.)
Engineering License Number: APEGA 164395



300, 6815 - 8 Street NE, Calgary, AB T2E 7H7 CANADA T 403 775-0100 F 403-775-0102

CIMA+ file number: Z0026600
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Table of involved resources

In addition to the signatories of this report, the following individuals have also been involved in the study and writing of the report as technical experts within the project team:

Name	Engineering License Number	Discipline
Pawan Bhattarai, P.Eng.	APEGA 235013	Infrastructure - Water Resources

Register of issues			
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Table of contents

1.	Introduction	1
1.1	Study Area	2
1.2	Scope and Objectives	3
1.3	Methodology	3
1.4	Approvals.....	4
2.	Stormwater Management Planning	5
2.1	Existing Drainage	5
2.1.1	Upstream Areas.....	5
2.2	Proposed Development.....	7
2.3	Existing Reports and Guidelines	10
2.4	Design Objectives	10
2.4.1	Stormwater Quality	10
2.4.2	Release Rate Control	11
2.5	Stormwater System Design	11
2.5.1	Major System	11
2.5.2	Minor System	12
2.5.3	Traplows.....	12
2.5.4	Pond Capacity	13
2.5.5	Best Management Practices	13
2.5.6	Geotechnical and Environmental Studies	14
2.5.7	Erosion Protection	15
2.5.8	Emergency Escape	15
3.	Methodology and Data	16
3.1	Computer Model	16
3.2	Hydrological Parameters	16
3.2.1	Infiltration	16
3.2.2	Depression Storage	17
3.2.3	Evaporation and Wind Speed	17
3.2.4	Snowmelt	17
3.2.5	Single Event Storm	17
3.2.6	Continuous Simulation	19
3.3	Pre-Development Analysis	19
3.3.1	Catchment Delineation and Flow Routing	19
3.4	Post-Development Analysis.....	20
3.4.1	Catchment Delineation and Flow Routing	20
3.4.2	Irrigation.....	20
3.4.3	Pond Sizing	21
3.4.4	Pond Discharge.....	21

4.	Analysis and Results	22
4.1	Pre-development	22
4.2	Post-development	22
4.2.1	Stormwater Quality	23
4.2.2	Pond Capacity	23
4.2.3	Release Rate	25
4.2.4	Runoff Volume	25
4.2.5	Emergency Escape	26
5.	Conclusion and Recommendations	27
5.1	Conclusion	27
5.2	Recommendations	28
6.	References	29

List of Tables

Table 2.1:	Pre-development catchments	6
Table 2.2:	Post-development imperviousness in developed areas	8
Table 3.1:	Horton infiltration parameters	16
Table 3.2:	Adjustments for soil conductivity for each month	17
Table 3.3:	Monthly average evaporation (mm/day)	17
Table 3.4:	Monthly average wind speeds (km/hr)	17
Table 3.5:	Snowmelt parameters	17
Table 3.6:	Preliminary pond stage-storage rating data	21
Table 4.1:	North and south streams peak flow	22
Table 4.2:	Post-development targets	22
Table 4.3:	Storage volume requirements for the proposed storm pond	23
Table 4.4:	Peak discharge into the north and south flows during the 100-year simulation	25
Table 4.5:	Total runoff volumes into the north and south streams in the continuous simulation	26

List of Figures

Figure 1-1:	Project Location within Rocky View County	1
Figure 1-2:	Site Location	2
Figure 2-1:	Pre-development catchments and existing drainage patterns	6
Figure 2-2:	Pre-development catchments and drainage patterns close to study area	7
Figure 2-3:	Draft post-development concept	8

Figure 2-4: Post-development catchments.....	9
Figure 3-1: 1-in-100-year design storm hyetograph	18
Figure 3-2: 1-in-500-year design storm hyetograph	18
Figure 3-3: Hourly precipitation data from 1960 to 2014.....	19
Figure 4-1: Visual goodness fit for Pearson III distribution graph using CoC Data and Frequency Sheet	24
Figure 4-2: Pond depth results from continuous simulation	24
Figure 4-3: Pond volume results from continuous simulation.....	25

List of Appendices

Appendix A Engineering Drawings

Appendix B Statistical Analysis

Appendix C PCSWMM Input and Output

1. Introduction

CIMA+ has been retained by Kinetikor Holdings #3 GP Ltd. to complete a stormwater drainage analysis of the Kinetikor land in Rocky View County in support of the Kinetikor Area Structure Plan (ASP). The Kinetikor lands is approximately 448 ha, located east of Balzac, north of Hwy 566, northeast of the City of Calgary. The legal land description is 14-26-28-W4, S ½ 23-26-28-W4M, and NE ¼ 15-26-28-W4M. The project location and site are shown in Figure 1-1 and Figure 1-2.

This Stormwater Master Drainage Plan (MDP) was prepared to provide guidance for the development of the Kinetikor land, including unit area allowable release rates (UARRs) and storage requirements for the site. This MDP adheres to the Rocky View County Servicing Standards (2025) and the City of Calgary Stormwater Management and Design Manual (2011).

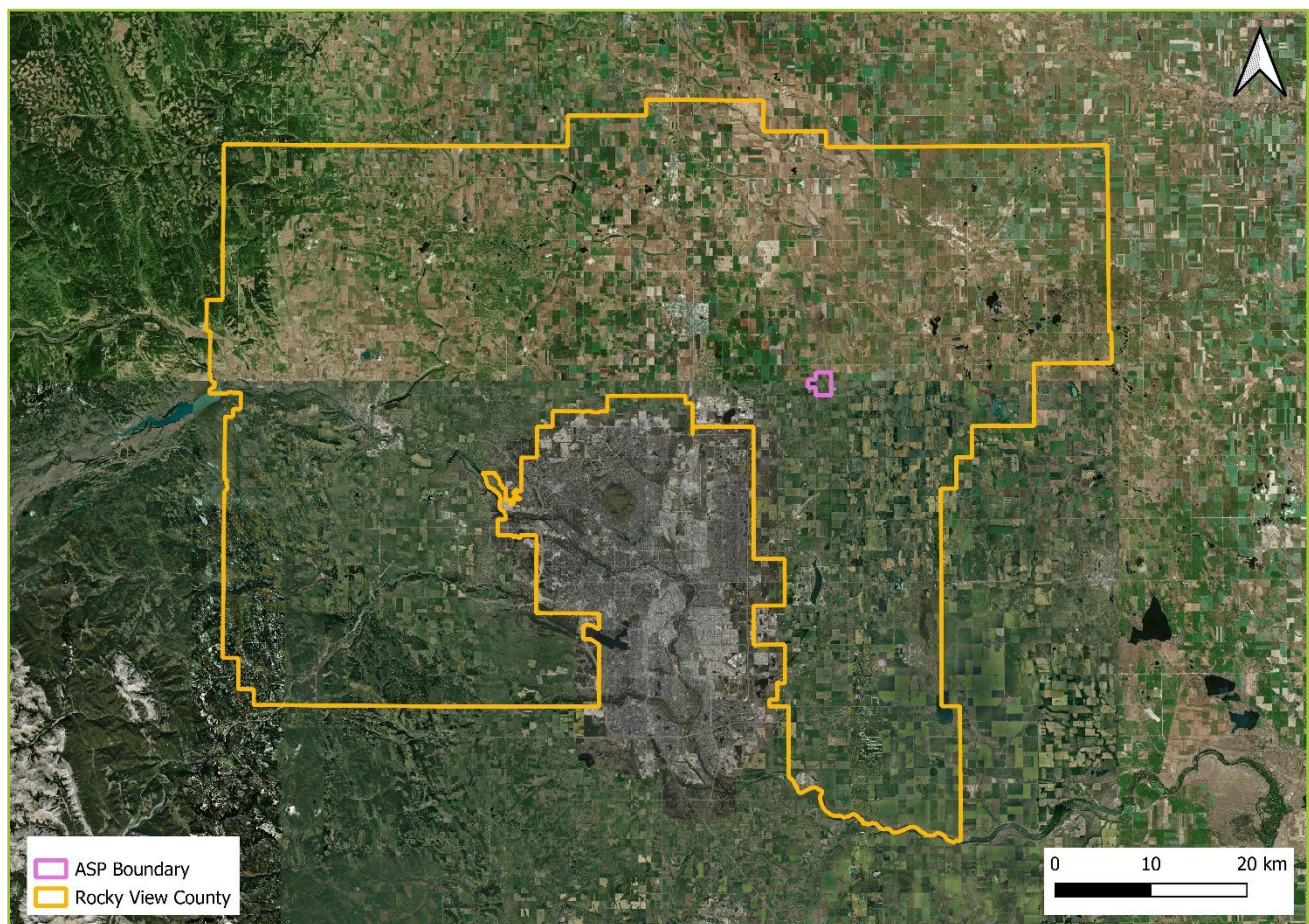


Figure 1-1: Project Location within Rocky View County

1.1 Study Area

The proposed Kinetikor ASP development encompasses approximately 448 ha, as shown in Figure 1-2 below. The area is located within Rocky View County and currently consists of undeveloped agricultural land. The proposed development, further described in the Kinetikor ASP prepared by CIMA+, consists of data center facilities, internal roads, a stormwater management facility (SWMF) to serve the development, and electrical grid interconnection and distribution facility.



Figure 1-2: Site Location

Note that while a small northeast portion of SE $\frac{1}{4}$ 23-26-28-W4M is not part of the ASP lands, it is included as part of the MDP study area because it naturally drains into the ASP lands. The total study area for this MDP is approximately 453 ha, including adjacent roads.

The existing site topography was evaluated using on site survey LiDAR survey data and contour maps. The study area generally drains from west to east and lies within the larger Rosebud River watershed. Drainage currently flows to two existing streams that traverse the site.

1.2 Scope and Objectives

Rocky View County requires a Master Drainage Plan (MDP) at Area Structure Plan (ASP) stage to support the development in Kinetikor development. The MDP will guide stormwater planning and ensure an effective and sustainable stormwater management system for current and future needs.

The more specific objectives of the MDP are as follows:

- Provide overarching guidance for both current and future stormwater management to ensure compliance with applicable provincial and municipal guidelines and regulations.
- Provide guidance on Low Impact Development (LIDs) and best management practices with respect to stormwater management, including stormwater use and water quality treatment.
- Define the pre and post development catchment boundaries, including upstream land and establish the outlet locations, release criteria for Kinetikor development as well as guidance for future developments within the catchment.
- Hydraulic analysis and modelling to estimate SWMF sizing requirement within the proposed Kinetikor ASP.

The MDP provides the overall design criteria and SWMF sizing for the study area. Further detailed stormwater analyses and reports will be completed as the development continues thorough neighbourhood area structure planning, subdivision planning and development stages adhering to the criteria and recommendation set in MDP.

No geotechnical, hydrogeological, structural, environmental, or geomorphological assessments were conducted as part of this study.

1.3 Methodology

The following methodology was used to conduct the study and prepare this MDP:

- Data collection and background study, review all available relevant information, documents, provincial and municipal guidelines.
- Establish drainage patterns and catchment areas based existing topography and conceptual outline plans.
- Based on the collected information, establish hydrological model parameters and design criteria for future development.
- Conduct pre- and post-development rainfall runoff analysis using hydrological model (PCSWMM) for single and continuous simulation to analyse pre-and post-development drainage conditions.
- Analyse the model results, summarize the results, and make recommendations.

1.4 Approvals

The study identifies the following requirements from the approving agencies:

- Approvals from the Rocky View County (RVC) and Alberta Environmental and Parks (AEP) under the Water Act and Environmental Protection Act will be required prior to development.

2. Stormwater Management Planning

To ensure adequate service levels for the development and to protect downstream infrastructure and watercourses, appropriate planning and design criteria must be established. Stormwater management for the Kinetikor ASP must address local topography while meeting Rocky View County's design standards, development objectives, and applicable regulations.

2.1 Existing Drainage

The study area is located within the Rosebud River drainage basin. Existing catchments consist of agricultural lands draining toward two local streams and roadway infrastructure (culverts and ditches). The northern stream discharges to the Graham Reservoir, while the southern stream drains to an unnamed pond that connects to the reservoir via a canal.

Based on flow contributions, the study area is divided into two drainage systems:

- North drainage area
- South drainage area

The southern drainage area collects runoff from the southern half of the study area. The northern half and western quarter drain toward the northern stream, as shown in Figures 2-1 and 2-2.

2.1.1 Upstream Areas

Figure 2-1 illustrates the drainage patterns of both streams. The southern stream originates within the study area, while the northern stream extends northwest beyond the study area. Stream extents and receiving water bodies were obtained from Alberta Base Waterbody data (Open Data portal). Elevation data was sourced from the High Resolution Digital Elevation Model (DEM) provided by the Government of Canada. Watershed delineation was performed using PCSWMM.

Catchments were grouped based on their contributing stream segment. Table 2.1 describes these catchments, and Figure 2-2 shows those closer to the study area. The total predevelopment study area is approximately 1,859 ha, including 448 ha within the ASP boundary.

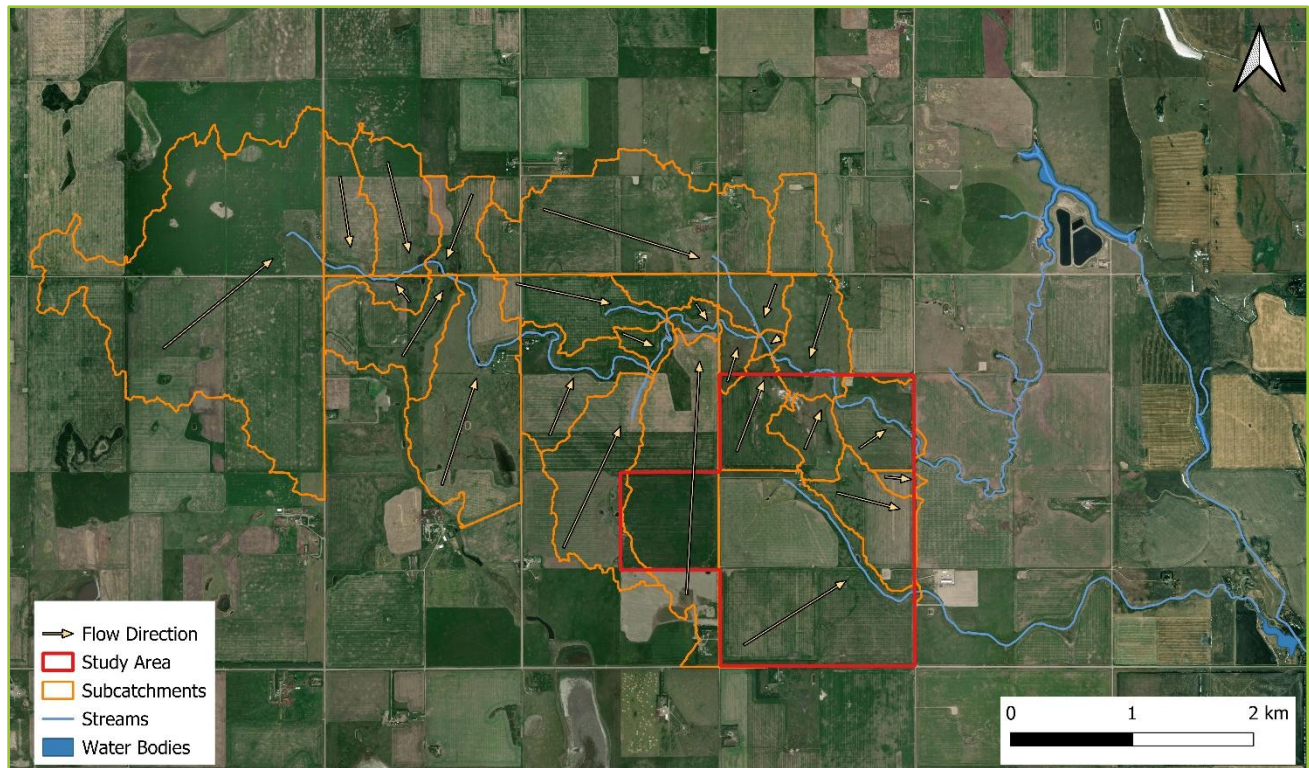


Figure 2-1: Pre-development catchments and existing drainage patterns

Table 2.1: Pre-development catchments

Catchment	Draining to Stream	Area (ha)	Slope (%)	Flow Length (m)
SC-01	South	207	4.6	1000
SC-02	North	51	3.7	500
SC-03	North	6	3.7	175
SC-04	North	45	5.4	475
SC-05	North	28	4.0	375
SC-06	North	85	3.9	650
SC-07	North	45	3.8	475
SC-08	North	4	6.2	150
SC-09	North	14	5.4	265
SC-10	North	45	4.0	475
SC-11	North	181	4.1	950
SC-12	North	11	5.5	235
SC-13	North	148	4.0	860
SC-14	North	96	3.0	690
SC-15	North	11	5.8	240
SC-16	North	54	4.2	520
SC-17	North	63	5.9	560

SC-18	North	139	4.2	835
SC-19	North	34	3.9	415
SC-20	North	65	4.5	570
SC-21	North	15	5.2	275
SC-22	North	57	4.6	535
SC-23	North	40	4.6	450
SC-24	North	412	4.2	1435

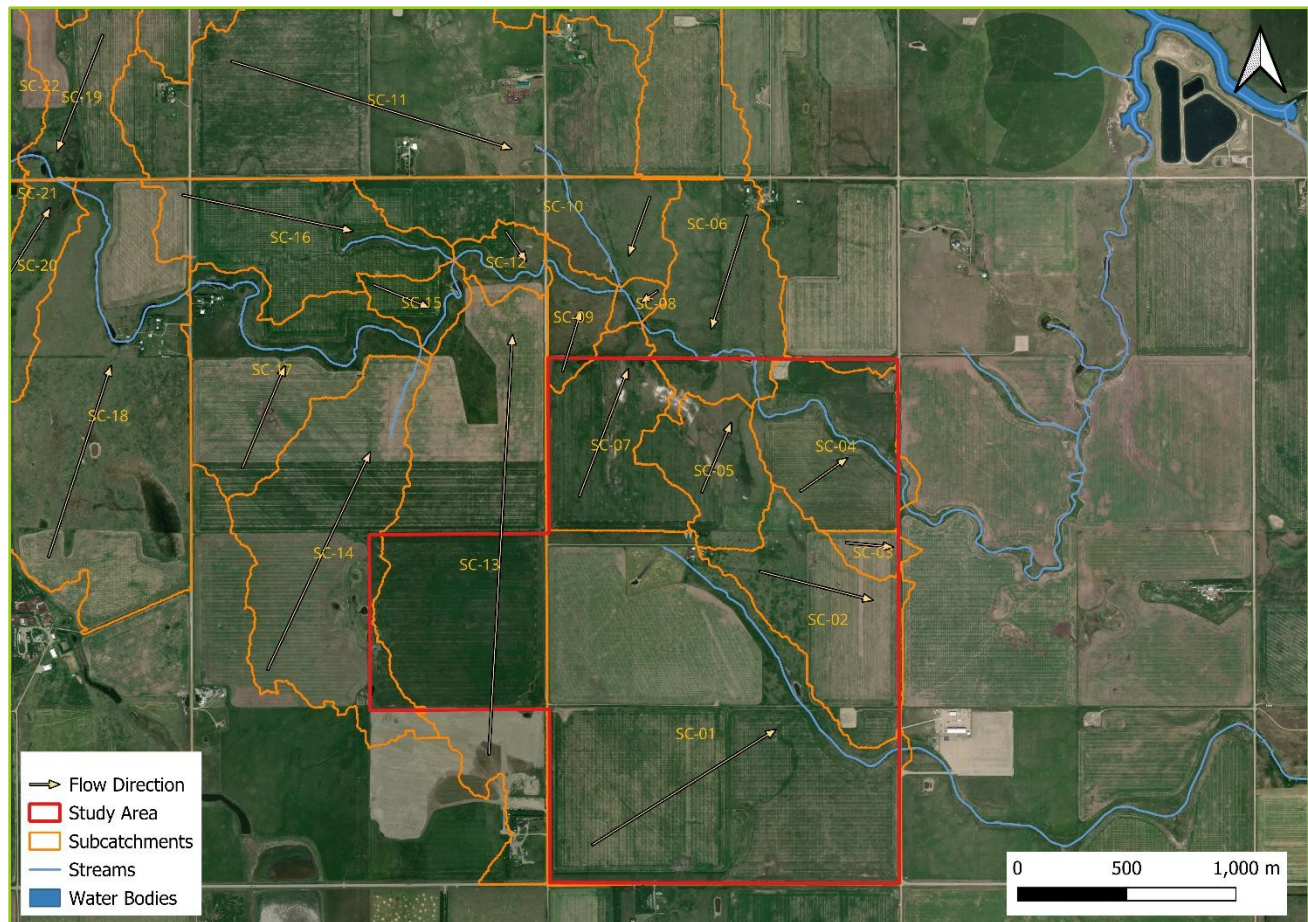


Figure 2-2: Pre-development catchments and drainage patterns close to study area

2.2 Proposed Development

The proposed ASP development includes data center buildings, internal roads, parking areas, and municipal reserve (MR) lands. A stormwater pond will be constructed on-site. The northeast portion of the study area will be retained in its pre-development condition for irrigation, supporting flow and volume control.

Post-development imperviousness for the developed area (approx. 373 ha) is estimated at 75.5%, as shown in Table 2.2.

Table 2.2: Post-development imperviousness in developed areas

Land Use*	Areas* (ha)	Imperviousness (%)	Impervious Area (ha)
Buildings	224	90	201.6
Internal roads, pavements, pathways	112	70	78.4
Municipal Reserve	37	5	1.8
Total Development Area	373	75.5%	281.8

* Estimated areas and land uses may be revised through detailed design

The storm pond is assumed as 100% impervious below the high water level (HWL), and 0% impervious above the HWL (grassed areas). The retained natural area is assumed as 0% impervious. Figures 2-3 and 2-4 show the draft post development concept and catchments respectively.



Figure 2-3: Draft post-development concept



Figure 2-4: Post-development catchments

Stormwater runoff from most developed areas (SC-1) will be conveyed to the storm pond via roadway ditches. Runoff from SC-2 may be routed via underground pipes due to grade constraints. Alternatively, SC-2 may be managed by a dedicated local stormwater facility prior to discharge to the northern stream.

For this MDP, SC-2 is assumed to discharge to the storm pond at the eastern ASP boundary to assess worst-case capacity. If a separate facility is designed for SC-2, it must meet the water quality and release rate targets defined in this MDP.

Runoff from the development will be routed through pervious areas where feasible, attenuated via traplow storage, and conveyed to the storm pond at an average peak rate of 90 L/s/ha.

The northern stream and adjacent natural lands will be retained post-development to allow upstream drainage and support stormwater irrigation (SC-3). The southern stream will be removed.

As shown in Figures 2-2 and 2-3, none of the external catchment areas will be impacted by ASP development. Only the southern portion of catchment SC-13, located southwest of the ASP, may be affected and re-routed via a ditch along the southwest boundary.

2.3 Existing Reports and Guidelines

Stormwater management within the ASP must comply with provincial and municipal regulations, including the following documents:

- Alberta Environment and Protected Areas Municipal Policies and Procedures Manual, 2024.
- Rocky View County Servicing Standards, 2025.
- Stormwater Management Guidelines for the Province of Alberta, 1999.
- The City of Calgary Stormwater Management & Design Manual, 2011.

No geotechnical, hydrogeological, structural, environmental, or geomorphological assessments were reviewed for this MDP. Recommendations from future studies should be incorporated where applicable.

2.4 Design Objectives

Two primary objectives were considered in the development of this MDP, namely stormwater quality enhancement and release rate control.

No runoff volume control targets were identified for this region.

2.4.1 Stormwater Quality

The Alberta Environment and Protected Areas Municipal Policies and Procedures Manual (2024) requires $\geq 85\%$ removal of sediments $\geq 75 \mu\text{m}$ prior to discharge. The City of Calgary Stormwater Management & Design Manual (2011) specifies a more stringent requirement of $\geq 85\%$ removal of sediments $\geq 50 \mu\text{m}$, which has been adopted for this MDP.

No additional water quality targets have been mandated.

The minimum detention time for the wet ponds is 24 hours. By controlling and treating all runoff generated by the more frequent events, represented by the water quality design event, the desired objectives should be achievable. The water quality storm event should follow Alberta Environment's guidelines of a 25 mm event.

2.4.2 Release Rate Control

The release rate is defined as the peak discharge from stormwater facilities during the 1:100-year, 24-hour City of Calgary design storm.

No specific release rate targets were identified for the study area in the reviewed documents (Section 2.3). Therefore, a pre-development analysis was conducted to establish baseline peak flow rates for setting post-development targets.

Based on this analysis (see Section 4.1), the following peak release rate targets were established:

- 11.2 L/s/ha to the north stream
- 10.7 L/s/ha to the south stream

These values apply for discharge from the storm pond at the eastern edge of the study area and are further detailed in Section 4.1.

2.5 Stormwater System Design

2.5.1 Major System

The major stormwater drainage system includes all overland drainage routes, such as swales, ditches, roads, and storage facilities. The major system facilitates flow when the minor system is restricted or beyond capacity; because of this, the major system is designed for infrequent extreme rainfall events that exceed the capacity of the minor system. Failure to adequately plan and design the major system can cause flooding and damage to public and private property during extreme events or minor system malfunctions.

Designing for the major system is required to follow Alberta Environment guidelines. In addition to the guidelines, a useful summary of the design standards has been prepared as part of the current Alberta Standards and Guidelines for Municipal Waterworks, Wastewater and Storm Drainage Systems (2013, Part 5). These documents outline all the requirements for stormwater management in Alberta; however, a few of the most pertinent guidelines regarding the major system are also listed below:

- The major drainage system is to be designed based on a 1:100-year rainfall event.
- This includes the sizing requirements of any stormwater management facilities.
- The development design must provide a continuous escape route for stormwater, typically via the road network. The emergency escapes towards adjacent properties must be evaluated and designed to ensure no adverse effects occur.
- The maximum depth of flow at curbside gutters should be less than 0.3 m.
- Standing water at low points (trap lows) should be less than 0.5 m and no more than 0.3m where possible.

- Velocities and flow depths for the major system, including gutters and swales, shall not exceed the permissible values prescribed by Alberta Environment.
- Sufficient freeboard must be provided between the water level along the major system and building elevations.

2.5.2 Minor System

The minor drainage system includes all the underground piping systems that facilitate transport of stormwater quickly and efficiently within its design capacity. Similar to the major system, the design guidelines for the minor system must follow Alberta Environment Guidelines; the minor system requirements can be found in the publications mentioned above.

Pertinent design guidelines for the minor system include:

- The minor system should be designed to carry the peak flow resulting from a 1:5-year rainfall event without surcharging.
- Storm system must be designed as a separate entity from the sanitary system.
- Storm sewer pipe shall be designed to convey the design flow with the hydraulic grade-line below the pipe crown (without surcharging). During detailed design of the drainage system, the hydraulic grade-line during the 1:100-year event must be acceptable. Surcharging to the surface should not be allowed. Inlet control devices (ICD) may be required to control flows into the piped system.
 - Surge in the pipe network should be a minimum of 1.2 m below surface in the worst-case scenario.
- Minimum depth of cover to the pipe crown should be minimum 1.2 m.
- Allowable release into the minor system should be between 100 L/s/ha where trap low storage is limited, and 70 L/s/ha where sufficient traplow storage can be provided.

Without any extensive precipitation data within the project boundary, it is recommended to use data from Calgary International Airport. This data is available on the City of Calgary's Water Resources webpage and in the *City of Calgary Stormwater Management and Design Manual*. The "Chicago Storm" distribution and Calgary's Intensity Duration Frequency (IDF) curve is recommended for design storms as it provides periods of high intensity rain; this information is found in Appendix K of the *City of Calgary Stormwater Management and Design Manual* and is already built into the PCSWMM software.

2.5.3 Traplows

Traplows are located at sags or low points near catch basins within the road or parks that prevent flow from cascading further downstream. The depression acts as storage and allows water to enter the minor system at a control rate, by controlling the inlet capacity of the catch basin grate or Inlet Control Devices (ICDs).

The traplow storage reduces the risk for downstream flooding by attenuating the flow from the major system and directing it into the storm sewer. Requirements for traplows include:

- Traplows must have a defined escape route.
- Maximum depth at trap lows should be 0.5 m; where possible, maximum ponding depth should be less than 0.3 m.
- Traplows should be sized to contain the 1:100-year flows.

2.5.4 Pond Capacity

The SWMFs will need to be adequately sized to attenuate the flow rate from the post development condition to the prescribed UARR. The attenuation can occur through multiple facilities or a single pond; however, the City of Calgary's guidelines suggest a minimum size of 2 ha at NWL for wet ponds. Servicing the development could be completed through wet or dry ponds, or constructed wetlands.

The volume requirement for attenuation is based on the greater of the 1:100-year design storm event or a frequency analysis using the continuous data set. Other requirements for storm ponds include:

- Permanent pool depth should be at a minimum of 2 m and a maximum of 3 m (wet ponds)
- Permanent pool volume should be a minimum of 25 mm over the catchment area multiplied by the imperviousness percentage (wet ponds)
- Maximum active storage depth should be 2 m (wet ponds)
- Maximum storage depth for water re-use should be 1.5 m (wet ponds)
- Minimum freeboard of 0.3 m (wet and dry ponds)

2.5.5 Best Management Practices

Direct runoff of stormwater from impervious surfaces, such as asphalt, to receiving waters can increase stream temperatures beyond the habitual limits of aquatic organisms. Fish species and aquatic invertebrates have temperature preferences that may be exceeded during periods of stormwater runoff. Vegetation around the storm ponds is recommended to help reduce the temperature of effluent stormwater. Other BMPs include implementation of lot level controls, outlet cooling, or SWMF configuration design to limit open areas of water.

Salt used on local roadways as de-icer in winter months can have a significant impact on stormwater quality during periods of snowmelt. Salt concentrations can have a negative impact on wildlife, freshwater ecosystems, vegetation, and soils. It is recommended that a salt management plan for the storage and use of road salts follows Environment Canada's Code of Practice for the Environmental Management of Road Salts.

Lot level best management practices (BMP) include source control practises, such as roof disconnections and 300 mm absorbent landscape.

2.5.5.1 Low Impact Development and Source Control

Controlling stormwater at the source is an effective way to reduce the demands on SWMFs and properly designed source controls can ultimately completely replace the need for SWMFs. LIDs aim to replicate pre-development hydrology, water balance, and provide water quality treatment through natural processes. Replicating the water balance is effective at reducing runoff volumes.

Further investigation would be required to determine the applicability and extent of LID design within the study area, especially identifying locations with permeable soils and the hydrogeological processes. A useful document outlining LID and other source control practices can be found in the City of Calgary's Stormwater Source Control Practices Handbook (2007).

The handbook outlines the various types of designs, source controls, and their applicability; however, a few of recommended practices are listed below:

- All roof drainage from single family homes and garages to be directed away from hard surface and towards landscaped areas prior to draining to streets or lanes. Rain barrels can also be used to intercept runoff.
- Absorbent landscapes with a minimum of 300 mm of topsoil.
- Vegetated swales

2.5.5.2 Stormwater Use

Stormwater use is currently proposed for the study area. The irrigation application details will need to be finalized through detailed design and will need to adhere to the applicable public health guidelines. The Alberta Health Services (AHS) *Public Health Guidelines for Water Use and Stormwater Use* (2021) outlines the applicable treatment needed before collected stormwater can be used for irrigation or other end uses.

2.5.6 Geotechnical and Environmental Studies

No geotechnical, hydrogeological, geomorphologic, or environmental assessments were available for review at the time of preparation of this MDP. At subsequent project phases, the stormwater analysis and outcomes from this MDP should be reviewed in conjunction with the relevant studies when available and their recommendations should be incorporated in future phases where possible.

2.5.7 Erosion Protection

The effect of increased runoff, even when controlled, could also cause increased erosion downstream of development. Although SWMFs attenuate the flows and keep them below a prescribed flow rate, duration and concentration of flow can still cause erosion problems in the channels downstream of a SWMF. Further studies at development stage will need to examine the erosion impacts and channel stability of downstream reaches and devise solutions to prevent erosion if necessary. Volume control can be considered as an option to assist in alleviating erosion concerns; however, an erosion assessment should still be undertaken during neighbourhood detailed design.

Concentration of flows to a single drainage path may also have erosion concerns. The use of berms or swales to direct flow or the installation of culverts may increase flow velocities. Appropriate erosion protection measures need to be provided at these locations.

2.5.8 Emergency Escape

The available capacity of the two streams downstream of the proposed development requires further analysis and should be undertaken when the development design progresses. This analysis should also investigate whether a practical emergency escape route from the Kinetikor Storm Pond is possible.

In the absence of a clearly defined emergency escape route, for this MDP, a 1-in-500-year design storm was also simulated to evaluate the pond's capacity. The results of this analysis are presented in Section 4.

3. Methodology and Data

In order to determine the pre-development and post development runoff conditions, computer modelling was used. Computer simulations can be completed for a single event or with continuous meteorological data over a period of time. Both scenarios were completed using City of Calgary data (1:100-year design storm event and 1960-2014 continuous data).

3.1 Computer Model

Computer aided PCSWMM model version 7.7.3920 (SWMM Version 5.2.4) was used for hydrological analysis.

PCSWMM is a software developed by Computational Hydraulic Inc. (CHI) that utilizes the base EPASWMM engine to simulate hydrological and hydraulic processes. PCSWMM is one of the modelling software packages accepted by the City of Calgary Stormwater Management and Design Manual (2011) and the Rocky View County Servicing Standards (2025).

The following section describes the hydrological parameters and methodology used in the model.

3.2 Hydrological Parameters

Hydrological parameter values such as monthly evaporation used in the model were obtained from the City of Calgary *Stormwater Management & Design Manual (2011)*.

3.2.1 Infiltration

The Horton approach was used for infiltration in this study. Since no field values were obtained for infiltration parameters, the soil infiltration parameters used in this model are based on the recommendations from the *City of Calgary Stormwater Management & Design Manual (2011)*.

Table 3.1: Horton infiltration parameters

Parameter	Value	Unit
Maximum Infiltration	75	mm/hr
Minimum Infiltration	7.5	mm/hr
Decay Constant	4.14	1/hr
Drying Time	7	days

To account for reduced infiltration during winter months due to frozen ground, a reduction factor shown in Table 3-2 was applied.

Table 3.2: Adjustments for soil conductivity for each month

JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
0.05	0.05	0.05	0.05	1	1	1	1	1	1	0.05	0.05

3.2.2 Depression Storage

The depression storage values used for this model are 1.6 mm for impervious surfaces and 3.2 mm for pervious surfaces post-development. For pervious surfaces in the pre-development condition, a depression storage of 5 mm was used to account for various low-lying areas. The Manning's 'n' values are 0.015 for impervious and 0.25 for pervious surfaces respectively.

3.2.3 Evaporation and Wind Speed

Average monthly evaporation and wind speed used in the continuous simulation are provided in Table 3.3 and Table 3.4 respectively.

Table 3.3: Monthly average evaporation (mm/day)

JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
0.10	0.38	1.12	2.40	3.61	4.57	4.99	4.00	2.24	0.99	0.27	0.07

Table 3.4: Monthly average wind speeds (km/hr)

JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
14.8	14.6	15.0	16.5	16.6	15.6	14.0	13.2	14.1	14.6	13.7	14.9

3.2.4 Snowmelt

To accurately account for snow buildup and snowmelt on site, the continuous simulation was set up with the snowmelt parameters shown in Table 3.5. For post-development impervious areas, snow plowing was included in the model using a coefficient of 0.3 for the fraction of impervious area that is plowable.

Table 3.5: Snowmelt parameters

Parameter	Value
Dividing temperature between snow and rain	0 °C
Minimum Melt Coefficient	0.05 mm/hr/°C
Maximum Melt Coefficient	0.20 mm/hr/°C
Fraction Free Water Capacity	0.10

3.2.5 Single Event Storm

The single event storm used in the model was the City of Calgary 1:100-year design storm. The design storm hyetograph is shown in Figure 3-1.

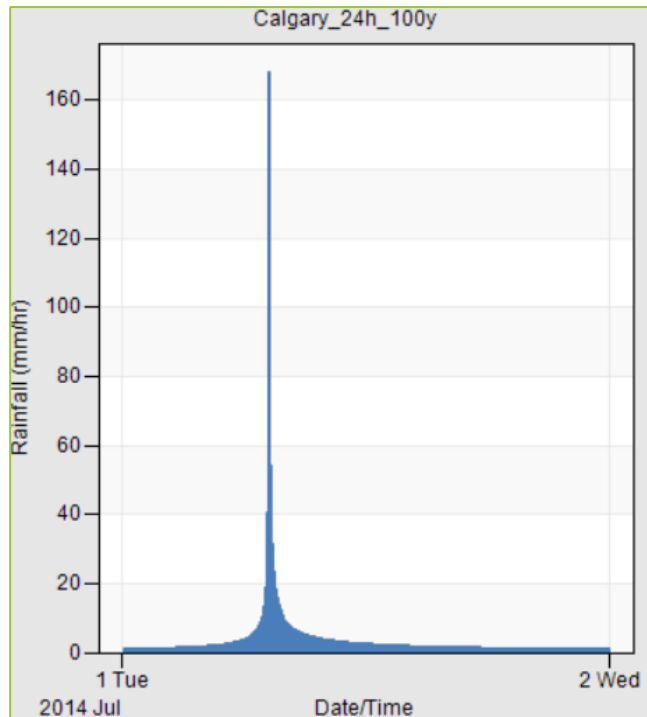


Figure 3-1: 1-in-100-year design storm hyetograph

In addition to the 1-in-100-year single event, the 1-in-500-year single event was also modelled to evaluate the capacity of the pond in the absence of an emergency escape, as per the City of Calgary Stormwater Management and Design Manual (2011). The 500-year storm hyetograph is shown in Figure 3-2.

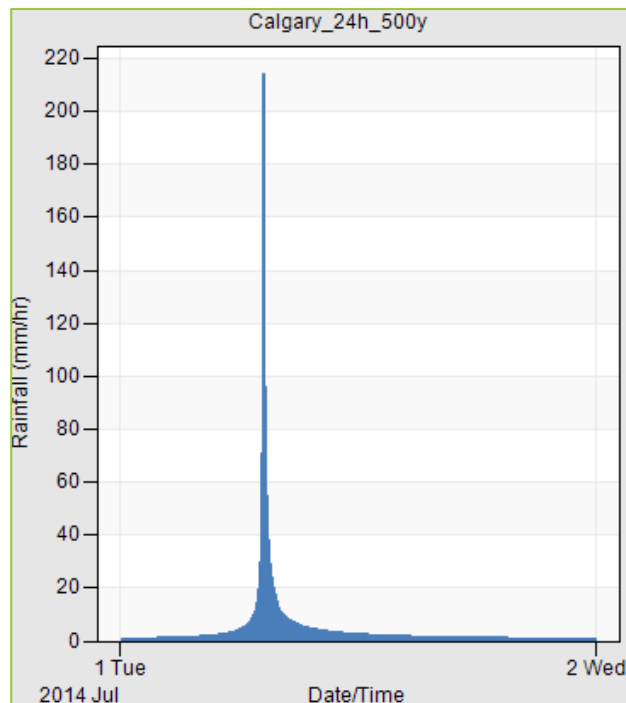


Figure 3-2: 1-in-500-year design storm hyetograph

3.2.6 Continuous Simulation

The continuous simulation precipitation data used in the model was obtained from the City of Calgary open data source. The precipitation data is shown in Figure 3-2.

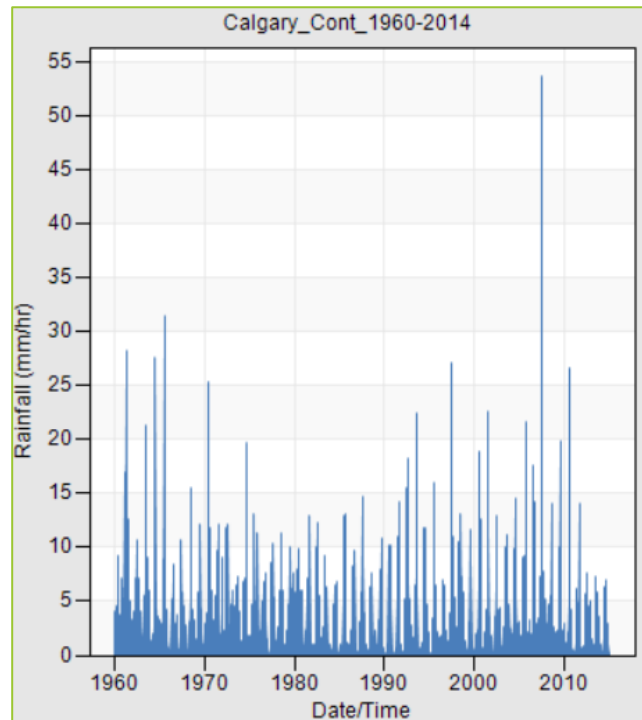


Figure 3-3: Hourly precipitation data from 1960 to 2014

3.3 Pre-Development Analysis

3.3.1 Catchment Delineation and Flow Routing

First, the existing pre-development drainage conditions were identified using onsite survey data gathered by CIMA+, waterbody features data obtained from the Alberta Open Data portal, and 1 m LiDAR elevation data from the High Resolution Digital Elevation Model (HRDEM) dataset from the Government of Canada.

The LiDAR and stream data were processed using the in-built watershed delineation tool on PCSWMM with an average catchment size of 1.0 hectare. A small catchment size was used to discretize the data and ensure that drainage boundaries were adequately captured. The smaller catchment areas were then manually combined based on the hydraulic routing outlet (section of the stream they contributed to) to determine the total pre-development catchments. The pre-development catchments are described in Section 2.1.

Note that local storage within the catchments, such as in depressions, ponds, and upstream of culverts was not considered. Culvert crossing data was not available. However, flow length, which represents the average length of overland sheet flow in a catchment before flow becomes channelized, was conservatively set high to increase the amount of infiltration losses in the catchment before entering the stream and thereby reduce the observed runoff. No flow data was available for the north and south streams for model calibration.

3.4 Post-Development Analysis

3.4.1 Catchment Delineation and Flow Routing

The post-development catchments were manually delineated based on the ASP concept and are shown in Figure 2-4. The flow lengths were estimated based on general lot sizes and distance between catch basins.

3.4.2 Irrigation

To consider for irrigation and stormwater use during the continuous simulation, it was assumed that 71.7 ha of predevelopment area in the northeast of the study area would use irrigation. The irrigation was included to account for the maximum pond volumes.

The irrigation demand is based on an assumed 4 mm/day and irrigation occurring through 153 days per year (May to September). The total demand per season is estimated below.

$$\text{Annual Irrigation Demand} = ((4 \text{ mm/day} \times 0.001 \text{ m/mm} \times 153 \text{ days} \times 71.7 \text{ ha} \times 10,000 \text{ m}^2/\text{ha}) \div 0.65) = 675,083 \text{ m}^3$$

This demand was then used to model the pumping rate needed in the model. The pump was assumed to be continuous at the single speed. Further detailed design of the pump system will need to be provided at the SMDP or pond report phase. For now, the pumping rate was calculated below. The pumping rate assumes that pond will provide water for 5 hours per day and 7 days per week.

$$\text{Pumping Rate} = 675,083 \text{ m}^3 \div (153 \text{ days} \times 5 \text{ hr/day} \times 3600 \text{ s/hr}) = 0.245 \text{ m}^3/\text{s}$$

Irrigation was set to occur when the pond water level elevations are between 2 m (lower normal water level, LNWL) and 4 m (0.5 m above the upper normal water level, UNWL) to utilize more of the stored volume in the pond for irrigation and reduce the overall discharge to the streams. Irrigation was turned off in the single event simulation.

3.4.3 Pond Sizing

To complete the modelling of the storm pond volumes, preliminary pond sizes were necessary. The pond curve used for this model was developed based on recommended depths and side slopes from the City of Calgary Stormwater Management & Design Manual (2011). Since irrigation was used in the continuous simulation, the pond employed a water use zone between the lower and upper normal water level (LNWL and UNWL).

The preliminary pond curve information is given in Table 3.6 below. The water level depth at the start of both the single-event and continuous simulations was set at the 2.75 m (halfway between the LNWL and UNWL). This is because the average depth in the pond through multiple iterations of the continuous simulation was found to be approximately 2.75 m.

Table 3.6: Preliminary pond stage-storage rating data

Stage	Elevation (m)	Depth (m)	Pond Area (m ²)	Incremental Storage (m ³)	Total Storage (m ³)	Active Storage (m ³)
Pond Bottom	1019.5	0.0	43,125	-	0	-
	1020.5	1.0	47,925	45,525	45,525	-
LNWL	1021.5	2.0	52,925	50,425	95,950	-
	1022.0	2.5	55,500	27,106	123,056	-
UNWL	1023.0	3.5	60,800	58,150	181,206	0
	1024.0	4.5	66,300	63,550	244,756	63,550
HWL	1025.0	5.5	72,000	69,150	313,906	132,700
Freeboard	1025.5	6.0	74,925	36,731	350,638	169,431

3.4.4 Pond Discharge

The primary means of discharge from the storm pond will likely be via one or more orifices located at different water level elevations in the pond's outlet control structure, and pumps. The final discharge regime should be finalized through detailed design with due consideration for the downstream receiving streams and waterbodies.

For this MDP, two conceptual discharge mechanisms were modelled:

- A 1.0 m wide × 0.4 m high rectangular orifice with invert elevation 1023.0 m (UNWL), directed to the north stream
- A pumped discharge to the south stream, ranging from 0.55 m³/s to a maximum of 2.2 m³/s, based on the water level elevation in the pond

The orifice and pump were sized such that the peak flows, when the main pond water level is at the HWL, do not exceed the targets set from the pre-development analysis (see Sections 2.4 and 4.1).

4. Analysis and Results

The results of the stormwater models are presented below for both pre-development and post-development conditions. The pre-development models were used to determine the release rate, while the post-development models were used to validate the preliminary pond sizing and drainage strategy against those targets.

4.1 Pre-development

Table 4.1 shows the estimated peak outflows from the north and south streams in the pre-development model at the eastern edge (outflow boundary) of the study area, where runoff from the study area and upstream catchments was routed into the streams. The post-development peak flow targets were set based on the results from the pre-development analysis and are summarized in Table 4.2.

Table 4.1: North and south streams peak flow

Stream	Total Contributing Area (ha)	Peak Flow (L/s)	Peak Flow (L/s/ha)
North	1651	18500	11.2
South	207	2212	10.7

Table 4.2: Post-development targets

Stream	Contributing within study area (ha)	Peak Flow (L/s/ha)	Peak Flow (L/s)
North	246	11.2	2,755
South	207	10.7	2,215
Total	453	11.0	4,970

4.2 Post-development

The post-development ASP concept is shown in Figure 2-3. A storm pond is proposed at the eastern edge of the study area to manage flows from approximately 381 ha of development, including the pond (SC-1, SC-2, and SC-POND). Approximately 72 ha in the northeast portion of the study area will be retained in the pre-development condition for stormwater irrigation (SC-3).

The north stream is proposed to be retained in its pre-development state to allow flow-through of runoff from upstream areas. The section of the south stream within the ASP will be removed.

Discharge from the storm pond will be delivered to two outfalls – one at the north stream and the other at the south stream at the boundary of the study area. The final discharge method, flow rates, and volumes will be confirmed during detailed design, based on the capacity of the receiving streams and downstream water bodies.

For this MDP, the retained lands (SC-3) were modelled as draining to the north stream by gravity. The uncontrolled peak flow rate from SC-3 is 1,221 L/s. Therefore, the gravity discharge orifice from the storm pond to the north stream was sized for a peak flow of 1,534 L/s. Similarly, the peak discharge from the pond to the south stream was facilitated through a pump control link, with a peak flow of 2,200 L/s.

4.2.1 Stormwater Quality

At this stage of study, the stormwater quality enhancement method has not been finalized. The pond should incorporate an oil-grit separator (OGS), a forebay, or an equivalent treatment system at the inlet to meet statutory water quality requirements (see Section 2.4). A detailed design of the OGS or forebay, including treatment efficiency, will be required at a later phase (Staged Master Drainage Plan or Pond Report).

4.2.2 Pond Capacity

Table 4.3 summarizes the performance of the proposed storm pond (see Section 3.3) during the 1-in-100-year design storm and the continuous simulation using hourly rainfall data from 1960 to 2014.

Table 4.3: Storage volume requirements for the proposed storm pond

Parameter	Value
Maximum total volume below HWL (m3)	313,906
Design HWL depth from pond bottom (m)	5.50
Pond volume during the 1-in-100-year design storm event (m3)	286,800
Peak depth in the pond during 100-year simulation (m)	5.12
Maximum pond volume during continuous simulation (m3)	307,900
Peak depth in the pond during continuous simulation (m)	5.42
Statistical 1-in-100-year volume (Pearson III) from continuous simulation (m3)	307,000
Depth corresponding to statistical 100-year volume (m)	5.40

4.2.2.1 Frequency Analysis

In addition to the computer models, the results of the continuous simulation were used to complete a statistical analysis, to estimate the 1:100-year storage volume requirement based on the continuous simulation results.

The annual volume data from the models were input into Hyfran and the City of Calgary's Data and Frequency Analysis Spreadsheet for analysis. The Pearson III distribution showed the best fit among all followed by Log Pearson III. Based on the results of the numerical and visual goodness-of-fit, the Pearson III distribution is chosen. The Pearson III distribution graph is shown in Figure 4-1.

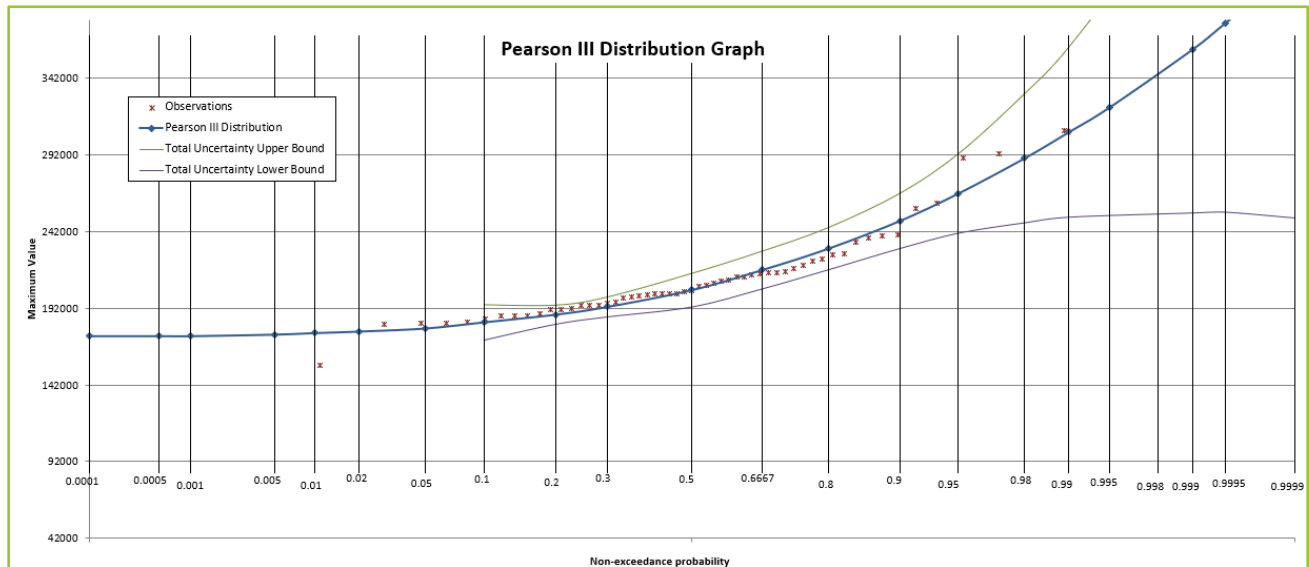


Figure 4-1: Visual goodness fit for Pearson III distribution graph using CoC Data and Frequency Sheet

More information regarding the data and frequency analysis for the pond can be found in Appendix E.

The total volumes from the 100-year event, the continuous simulation, and statistical 100-year event are less than the pond's total storage volume. Therefore, the pond is adequately sized to contain runoff from these events.

Figure 4-2 and Figure 4-3 show the pond depth and volume results from the continuous simulation for reference.

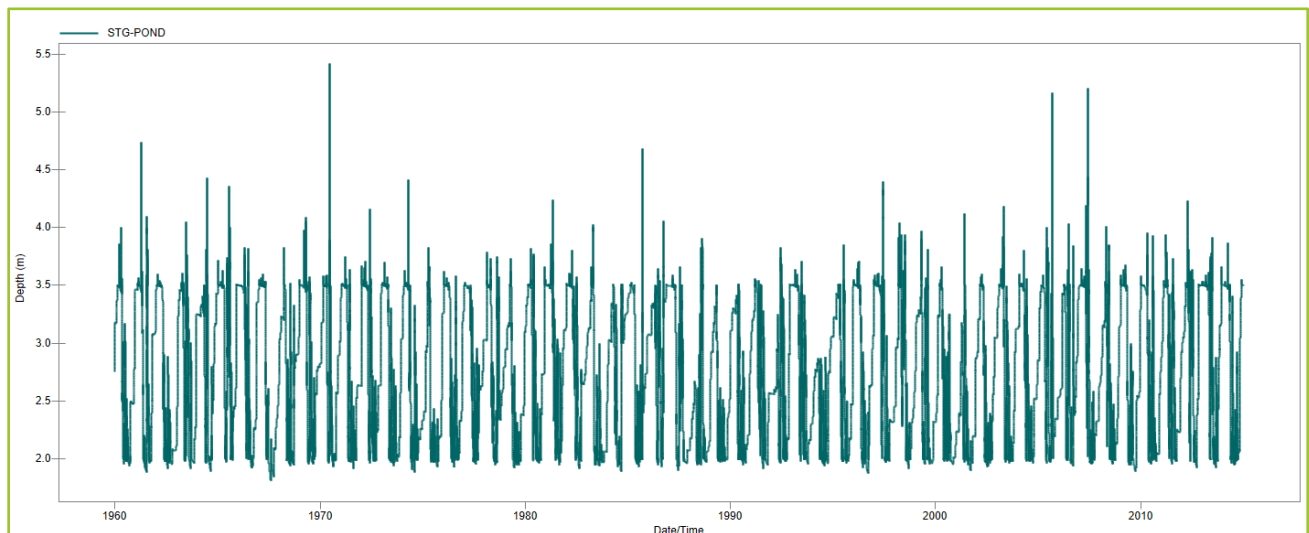


Figure 4-2: Pond depth results from continuous simulation

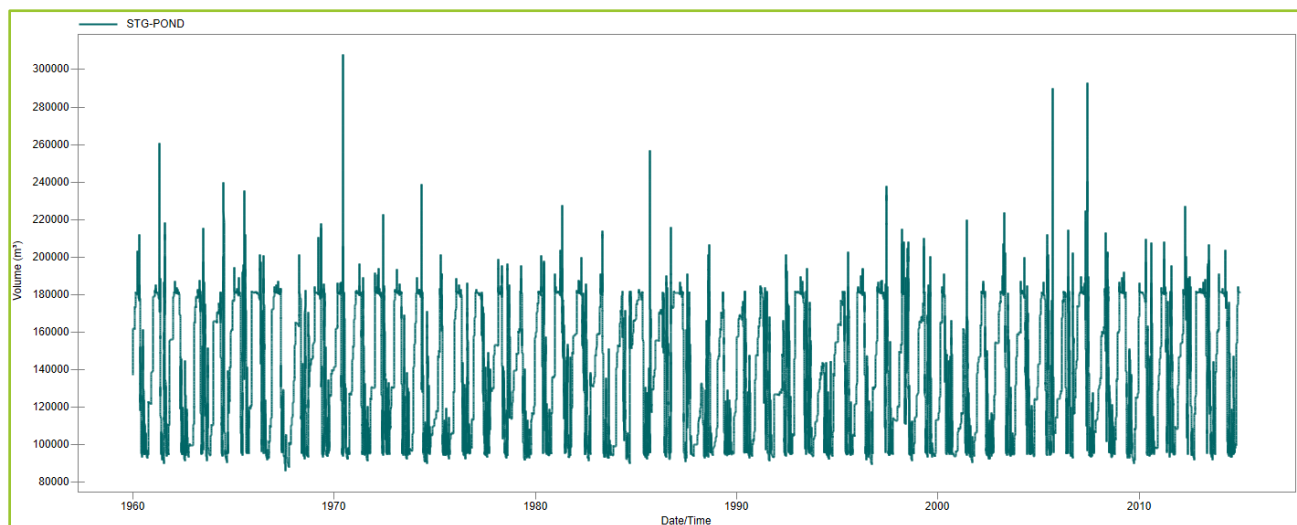


Figure 4-3: Pond volume results from continuous simulation

4.2.3 Release Rate

The peak flows to the north and south streams from the 100-year simulation are shown in Table 4.4.

Table 4.4: Peak discharge into the north and south flows during the 100-year simulation

Outfall	Stream	Allowable Peak Flow (L/s)	Observed Peak Flow (L/s)
OF-N-1 (Natural area SC-3 to north stream)	North	2,755	1,221
OF-N-2 (Gravity discharge from pond to north stream)	North	2,755	1,336
OF-S-1 (Pumped discharge from pond to south stream)	South	2,215	2,200
Total		4,970	4,757

The total peak discharges to the north and south streams are within the allowable limits, and therefore, the release rate targets have been met.

4.2.4 Runoff Volume

While no runoff volume target was adopted for the study area, the City of Calgary Industry Bulletin – Update for Watersheds with Voluntary Targets (2018) suggests a voluntary runoff volume target of 40 mm for development areas outside the Nose Creek watershed, Pine Creek watershed, and the Western Headworks Canal direct discharge area.

The study area is located outside the above noted watersheds, and therefore, a voluntary 40 mm annual average release volume (AARV) target was considered. The total offsite runoff from the study area during the continuous simulation is provided in Table 4.5.

Table 4.5: Total runoff volumes into the north and south streams in the continuous simulation

Outfall	Stream	Contributing Area within study area (ha)	Total Volume (m3)	AARV over 55 years (mm/year)
OF-N-1 (Natural area SC-3 to north stream)	North	246	166,200	1.23
OF-N-2 (Gravity discharge from pond to north stream)	North	246	1,396,500	10.32
OF-S-1 (Pumped discharge from pond to south stream)	South	207	6,819,000	59.89
Total		453	8,381,700	33.64

4.2.5 Emergency Escape

Since a viable emergency escape route has not been identified, the 1-in-500-year design storm was simulated. The pond was found to flood under this condition. The pond can be modified to contain this event. Alternatively, a suitable emergency escape route should be identified and analyzed to ensure sufficient capacity for events exceeding the 1-in-100-year design event.

5. Conclusion and Recommendations

The proposed development of the Kinetikor ASP lands in Rocky View County includes approximately 448 ha of land, with 381 ha designated for development and 72 ha retained in a pre-development condition for stormwater irrigation with total study area of 453 ha. The study area drains to two natural streams, one to the north and one to the south. The north stream drains directly to Graham Reservoir, while the south stream drains to an unnamed pond that is also connected with the Graham Reservoir via a canal. The south stream within the ASP boundary will be removed, while the north stream will be retained to convey upstream flows.

Stormwater runoff from the developed areas will be managed through a stormwater management facility (SWMF) located at the eastern edge of the study area. The SWMF will attenuate post-development peak flows to match pre-development conditions, with target release rates of 11.2 L/s/ha to the north stream and 10.7 L/s/ha to the south stream. The facility will also support stormwater use through irrigation of the retained lands.

Hydrologic modelling was completed using PCSWMM for both single-event (1:100-year and 1:500-year) and continuous simulations (1960–2014). The pond was sized to accommodate runoff volumes from both simulations. A 1:500-year event was also modelled to assess emergency escape needs, and the pond was found to flood under this condition, indicating that a defined emergency escape route is required.

Stormwater quality treatment will be provided through an oil-grit separator (OGS), forebay, or equivalent system, to meet the 85% TSS removal target for particles $\geq 50 \mu\text{m}$, in accordance with the City of Calgary guidelines. Further design of the treatment system will be completed at the SMDP or pond report stage.

Low Impact Development (LID) practices and best management practices (BMPs) such as absorbent landscaping, vegetated swales, and roof disconnection have been identified as potential strategies to reduce runoff volume and improve water quality. A voluntary annual average release volume (AARV) target of 40 mm was considered based on the watershed location.

5.1 Conclusion

The stormwater management strategy for the Kinetikor ASP lands has been developed in accordance with the Rocky View County Servicing Standards (2025) and the City of Calgary Stormwater Management and Design Manual (2011). The proposed SWMF is preliminarily sized to meet peak flow and volume control requirements under both single-event and continuous conditions. The strategy includes provisions for stormwater use, water quality treatment, and future integration of LID measures.

Further work is required at the SMDP and detailed design stages to:

- Finalize the pond outlet structure and discharge configuration.
- Confirm the capacity of downstream receiving streams.
- Identify and design a viable emergency escape route.
- Complete detailed design of the stormwater quality treatment system.

5.2 Recommendations

The following recommendations are made to guide future phases of stormwater management planning and design for the Kinetikor ASP lands:

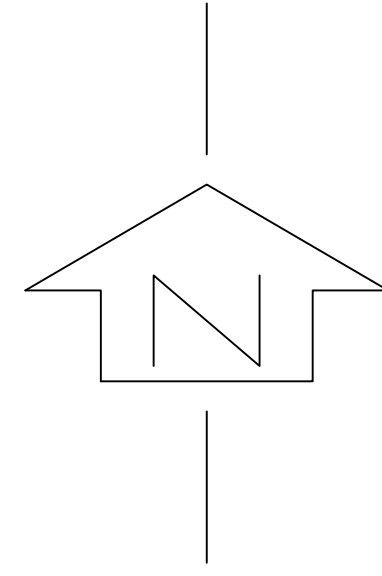
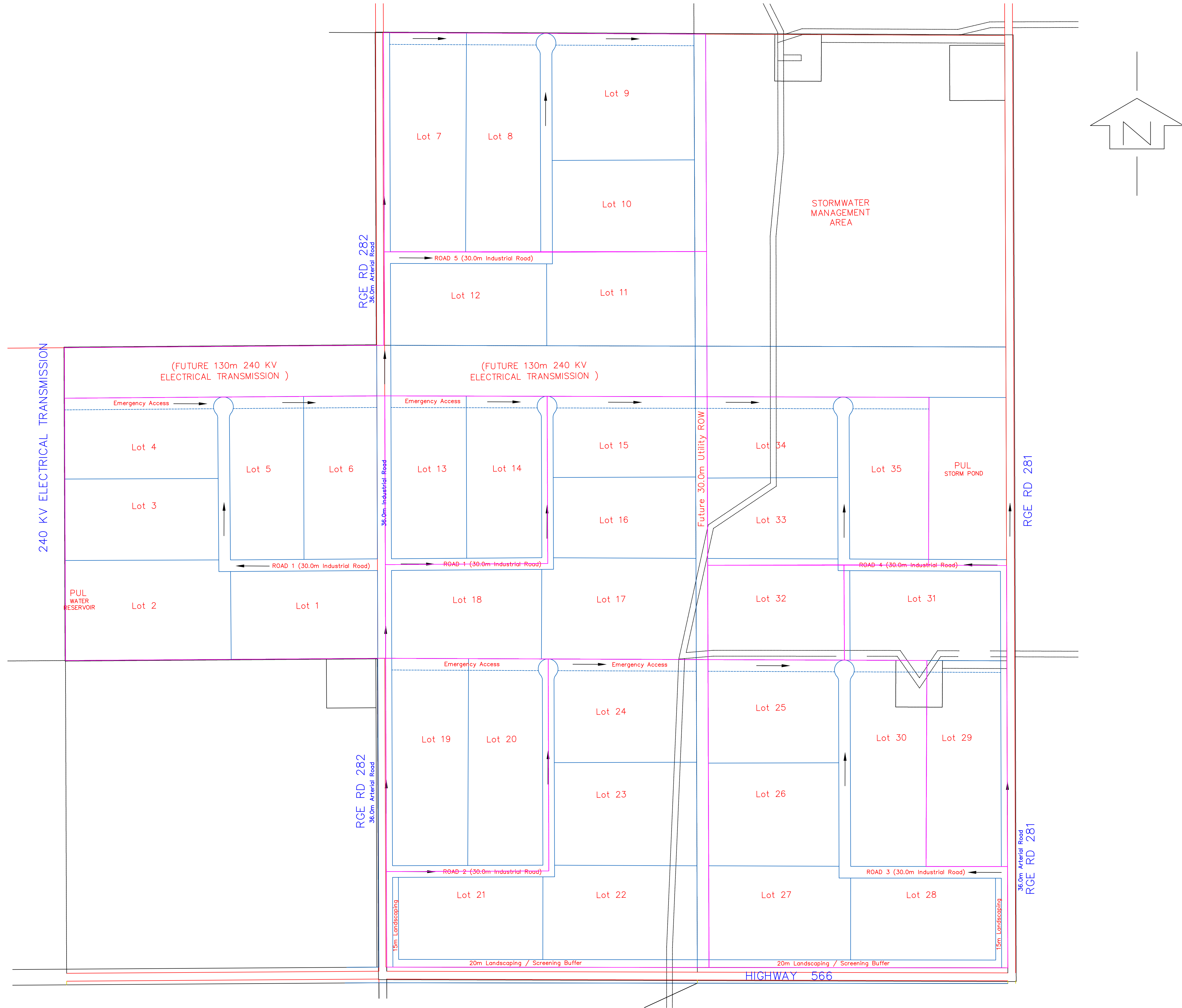
- Confirm the final release rates through detailed design to ensure compliance with the pre-development targets of 11.2 L/s/ha (north stream) and 10.7 L/s/ha (south stream).
- Finalize the storm pond outlet configuration, including orifice and pump design, based on downstream capacity and operational requirements.
- Identify and design a viable emergency escape route to safely convey flows during events exceeding the 1:100-year design storm.
- Complete detailed design of the stormwater quality treatment system, including oil-grit separators (OGS), forebays, or equivalent technologies, to meet the 85% TSS removal target for particles $\geq 50 \mu\text{m}$.
- Incorporate Low Impact Development (LID) and Best Management Practices (BMPs) such as absorbent landscaping, vegetated swales, and roof disconnection to reduce runoff volume and improve water quality.
- Confirm the feasibility of stormwater use for irrigation, including pump system design and compliance with Alberta Health Services (AHS) guidelines for water use.
- Integrate findings from future geotechnical, hydrogeological, and environmental studies to refine stormwater infrastructure design and assess the suitability of LID practices.
- Verify downstream conveyance capacity of the north and south streams and assess potential impacts of concentrated flows or erosion at discharge locations.
- Update the stormwater model as the ASP concept plan is refined and neighbourhood-level planning progresses, ensuring alignment with Rocky View County and City of Calgary standards.

6. References

- Alberta Environment and Parks. (2010). Base Waterbody Polygon Arc. Retrieved from <https://open.alberta.ca/dataset/gda-a2216d84-77ab-4f65-ab34-03434442cf32>
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A

Appendix A Engineering Drawings



KINETICOR

ROCKY VIEW COUNTY, ALBERTA

PRELIM DESIGN

STORM CATCHMENTS AREAS

LEGEND

DATE: JULY 17, 2025	SCALE: 1:5000
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PROJECT No.: Z0026800	CUTFILL PLAN
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LEGEND

MAJOR CUT CONTOUR 0.50 METRE INTERVAL

MINOR CUT CONTOUR 0.10 METRE INTERVAL

MAJOR FILL CONTOUR 0.50 METRE INTERVAL

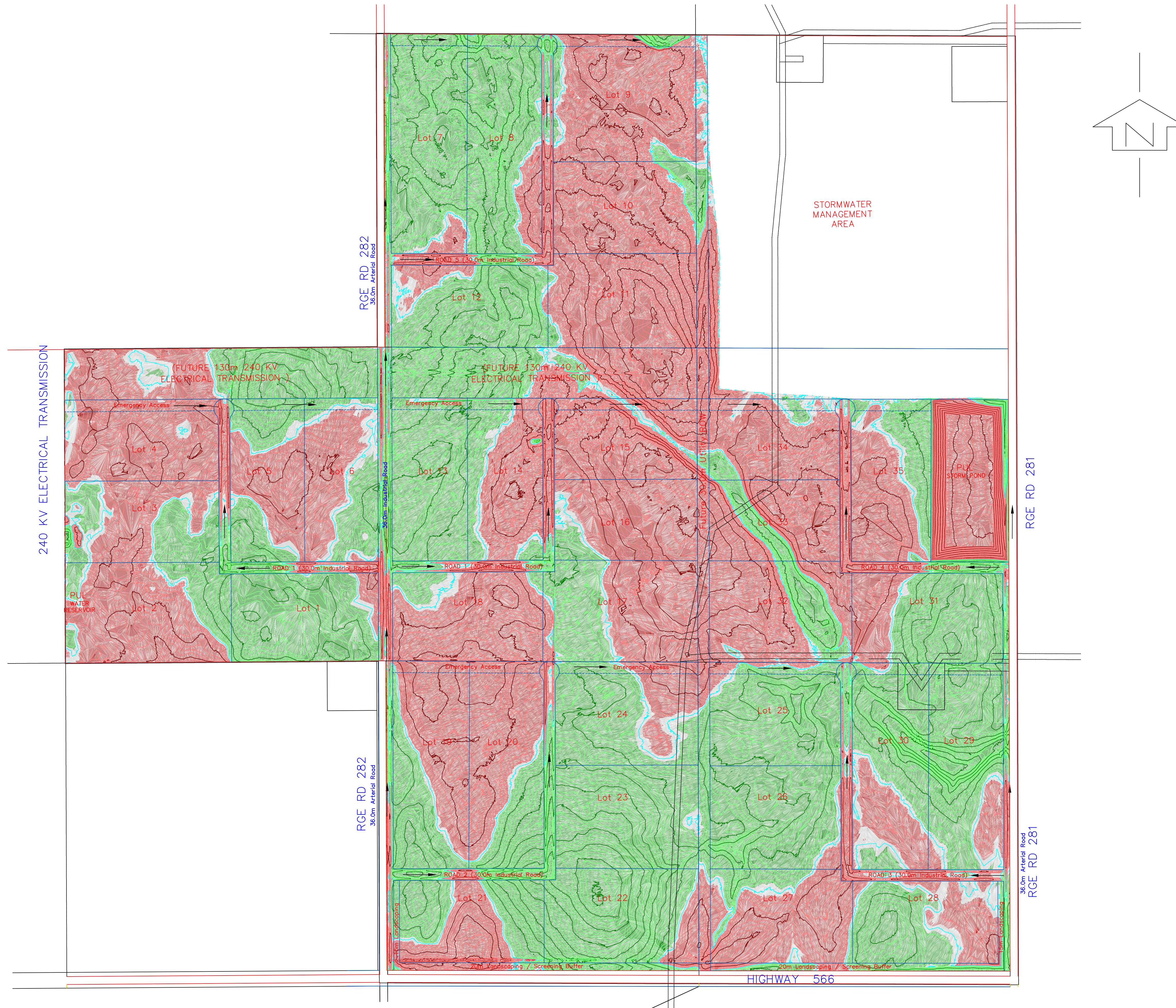
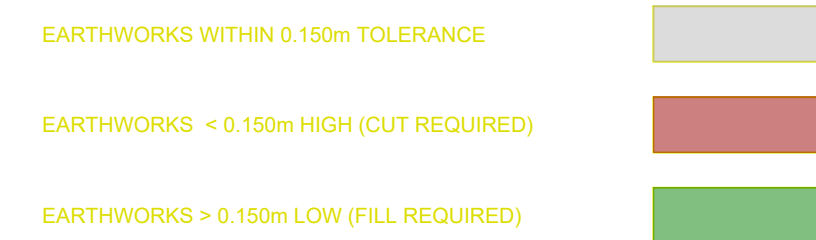
MINOR FILL CONTOUR 0.10 METRE INTERVAL

ZERO CONTOUR



The legend illustrates five types of contour lines: 1. Major Cut Contour (0.50 metre interval): A thick red line with a '0.50' label. 2. Minor Cut Contour (0.10 metre interval): A thin red line with a '0.50' label. 3. Major Fill Contour (0.50 metre interval): A thick green line with a '0.50' label. 4. Minor Fill Contour (0.10 metre interval): A thin green line with a '0.50' label. 5. Zero Contour: A thick blue line with a '0.00' label.

MAJOR CUT CONTOUR 0.50 METRE INTERVAL
MINOR CUT CONTOUR 0.10 METRE INTERVAL
MAJOR FILL CONTOUR 0.50 METRE INTERVAL
MINOR FILL CONTOUR 0.10 METRE INTERVAL
ZERO CONTOUR



Name	Cut Factor	Fill Factor	2d Area	Cut	Fill	Net
VOL-OG vs DG	1.000	1.000	3810301.75sq.m	2970623.57 Cu. M.	3026816.06 Cu. M.	56192.49 Cu. M.<Fill>
Totals			3810301.75sq.m	2970623.57 Cu. M.	3026816.06 Cu. M.	56192.49 Cu. M.<Fill>

Name	Cut Factor	Fill Factor	2d Area	Cut	Fill	Net
VOL-OG vs DG	1.000	1.000	3810301.75sq.m	2970623.57 Cu. M.	3026816.06 Cu. M.	56192.49 Cu. M.<Fill>
Totals			3810301.75sq.m	2970623.57 Cu. M.	3026816.06 Cu. M.	56192.49 Cu. M.<Fill>

B

Appendix B Statistical Analysis

DFASCC

Data and Frequency Analysis Spreadsheet for the City of Calgary
Version 1.2

PROJECT INFORMATION SHEET

Project Name:	Kineticor Master Drainage Plan
Project Description:	Stormwater MDP and conceptual pond for Kineticor ASP
Location:	Rocky View County
Date:	2025-07-22
Designed by:	Ajay Muthukumar
Company Name:	CIMA+
Reviewed by:	JF Chenier

Clear Project
Information Sheet

Hydrologic Data Series Input

NOTES

- This Spreadsheet is designed for a maximum of 1,000 entries (if more are required then formulas need to be adjusted)
- Input dataset must be based on uniform time distribution (i.e.: daily, weekly, monthly, yearly) and must not include multiple values for any of the time steps (duplicates highlighted in red)
- Input dataset must not have any missing cells of data (cells with "0" will be treated as having a value of 0)
- Only positive values should be used
- Please refer to **Section 2.2 of the Frequency Analysis Procedure for Stormwater Design Manual** for Data Series Characteristics detailed descriptions.

[illegible]

Clear All Input Data

Basic Characteristics	
Number of Data Entries	55
Maximum Value	308000
Minimum Value	155000
Average (Mean) Value	211000
Median Value	204000
Standard Deviation	28500
Variance	812000000
Variation coefficient (Cv)	0.135
Skewness coefficient (Cs)	1.53
Kurtosis	5.49

*Values assumed to be sample not full population

Empirical Probability of Non-Exceedance (Plotting Position) based on:

$$F(x(k)) = (k-a)/(n-2a+1), \quad 0 \leq a \leq 0.5$$

a = 0.4 Cunnane (1978)

k = rank of the even in question (in ascending order)

n= 55

Summary Sheet									
Initial Statistical Tests:				Project Information					
Tests for Stationarity									
Test		Result			Project Name:		Kineticor Master Drainage Plan		
Spearman Rank Order Correlation Coefficient		No Significant Trend at 0.05 Significance Level			Project Description:		Stormwater MDP and conceptual pond for Kineticor ASP		
Mann-Whitney Test for jump (a.k.a. Mann-Whitney U test)		No Jump at 0.05 Significance Level							
Wald-Wolfowitz Test (The runs test)		No Jump at 0.05 Significance Level							
Tests for Homogeneity									
Test		Result			Location:		Rocky View County		
Mann-Whitney Test for jump (a.k.a. Mann-Whitney U test)		Sample is Homogeneous at 0.05 Significance Level			Date:		2025-07-22		
Terry Test		Sample is Homogeneous at 0.05 Significance Level			Designed by:		Ajay Muthukumar		
Tests for Independence				Company Name:					CIMA+
Test		Result			Reviewed by:		JF Chenier		
Spearman Rank Order Correlation Coefficient		Data is independent at 0.05 Significance Level							
Wald-Wolfowitz Test for Independence		Data is independent at 0.05 Significance Level							
Anderson Test		Data is independent at 0.05 Significance Level							
Test for Outliers									
Test		Result							
Grubbs and Beck Test for Outliers									
Are any high outliers present?		High Outlier May Be Present							
Are and low outliers present?		No Low Outliers Present							
Numerical Goodness-of-fit Tests Results									
Distribution Type	Numerical Goodness-of-fit Tests from Spreadsheet			Average of Ranks	Ranking from Numerical Tests	Numerical Goodness-of-fit Tests from Hyfran (Input by user)		Notes from Visual Goodness-of-fit Test	
	A-D Test	K-S Test	Least Squares Ranking			BIC	AIC		
Normal	7	7	7	7.00	8				
Lognormal	6	6	6	6.00	6				
Lognormal III	4	5	5	4.67	5				
Exponential	9	9	9	9.00	10				
Pearson III	1	3	2	2.00	1				
Log Pearson III	5	1	1	2.33	2				
Gumbel	3	4	3	3.33	4				
GEV	2	2	4	2.67	3				
Weibull	8	8	8	8.00	9				
Gamma		10	10	6.67	7				

Selected Distribution and Results

Distribution type chosen based on visual and numerical goodness-of-fit tests:

Pearson III

Instructions:
- Based on the results of the numerical and visual goodness-of-fit tests presented above, choose the preferred distribution in the cell on the left

Return Period	Probability	Magnitude	Total Uncertainty (Upper Bound)	Total Uncertainty (Lower Bound)
10000	0.9999	415000	579000	251000
2000	0.9995	378000	502000	255000
1000	0.9990	361000	468000	254000
500	0.9980	345000	437000	254000
200	0.9950	323000	393000	253000
100	0.9900	307000	363000	251000
50	0.9800	290000	332000	248000
20	0.9500	267000	293000	241000
10	0.9000	249000	267000	231000
5	0.8000	231000	245000	217000
3	0.6667	217000	229000	205000
2	0.5000	204000	215000	193000
1.4286	0.3000	193000	200000	187000
1.25	0.2000	188000	194000	182000
1.1111	0.1000	183000	195000	172000
1.0526	0.0500	179000	#N/A	#N/A
1.0204	0.0200	177000	#N/A	#N/A
1.0101	0.0100	176000	#N/A	#N/A
1.005	0.0050	175000	#N/A	#N/A
1.001	0.0010	174000	#N/A	#N/A
1.0005	0.0005	174000	#N/A	#N/A
1.0001	0.0001	174000	#N/A	#N/A

*Total uncertainty is based on sampling uncertainty at ((95%) Confidence Interval) plus distribution uncertainty of Top 4 distributions (based on numerical goodness of fit tests)

Pearson III Distribution Graph

Maximum Value

Non-exceedance probability

Observations

Pearson III Distribution

Total Uncertainty Upper Bound

Total Uncertainty Lower Bound

Errors and Warnings

Cumulative distribution function warning

No warning

No warning

No warning

No warning

CDF based on parameters does not match Pearson III distribution

No warning

No warning

No warning

No warning

No warning

If a warning is present, please check if hyfran output results were pasted correctly. If hyfran results were pasted correctly the warning signifies that the Continuous Distribution Function (CDF) used in this workbook does not produce same output values as the input frequency analysis results, which in turn indicates that the numerical goodness-of-fit tests calculated by this spreadsheet for this distribution may be based on inaccurate numbers. Another possible solution would be to use a different method of estimating the CDF parameters for example: method of weighted moments.

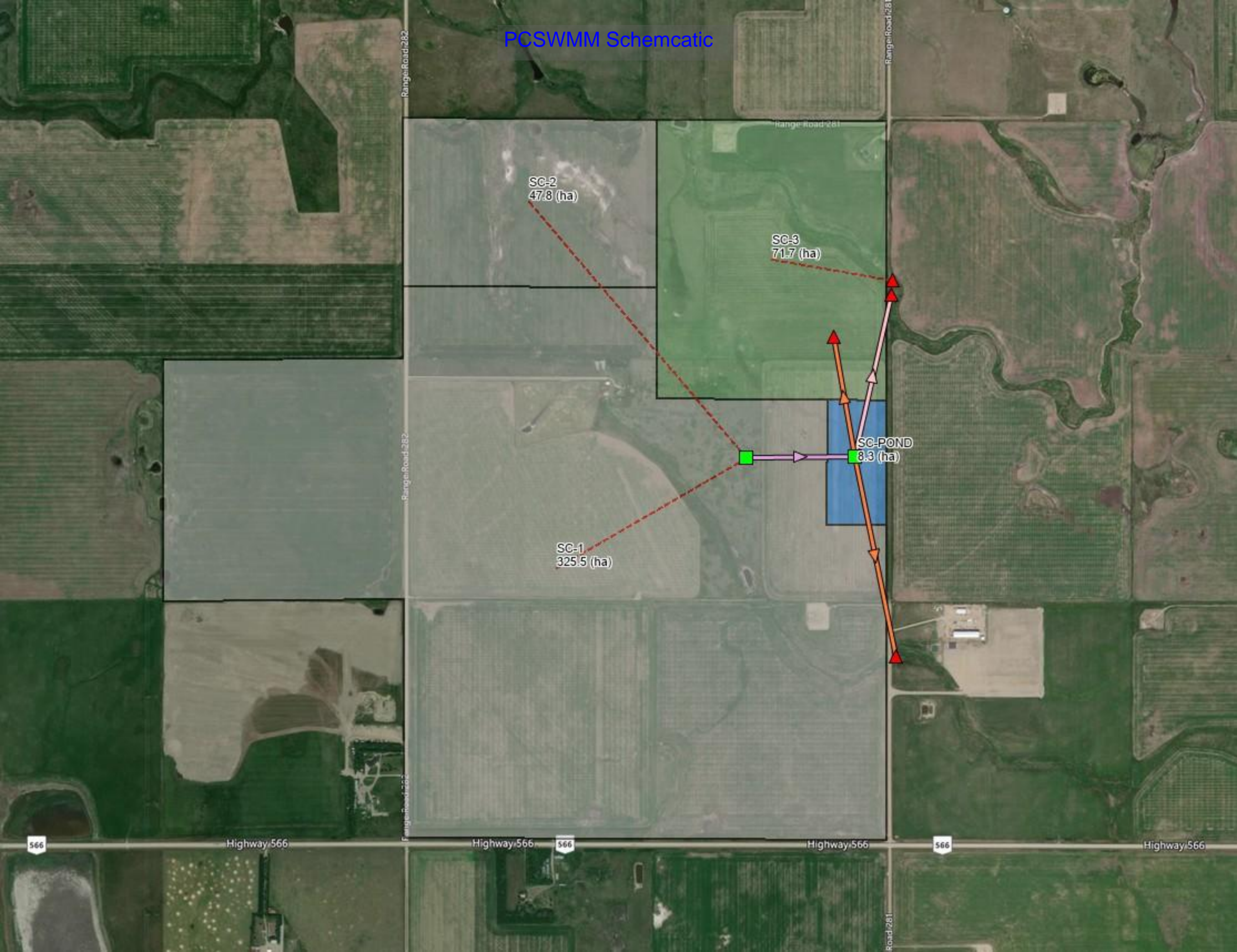
2 of 2

Company Name: CIMA +

C

Appendix C PCSWMM Input and Output

POSWMM Schematic



[TITLE]

;;Project Title/Notes

[OPTIONS]

```
;;Option      Value
FLOW_UNITS    CMS
INFILTRATION  HORTON
FLOW_ROUTING  DYNWAVE
LINK_OFFSETS  ELEVATION
MIN_SLOPE     0
ALLOW_PONDING NO
SKIP_STEADY_STATE NO
```

```
START_DATE    07/01/2014
START_TIME    00:00:00
REPORT_START_DATE 07/01/2014
REPORT_START_TIME 00:00:00
END_DATE      07/07/2014
END_TIME      23:00:00
SWEEP_START   01/01
SWEEP_END     12/31
DRY_DAYS      0
REPORT_STEP   00:05:00
WET_STEP      00:00:05
DRY_STEP      00:00:05
ROUTING_STEP  5
RULE_STEP     00:00:00
```

```
INERTIAL_DAMPING  PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP     0.75
LENGTHENING_STEP  0
MIN_SURFAREA      0
MAX_TRIALS         8
HEAD_TOLERANCE     0.0015
SYS_FLOW_TOL       5
LAT_FLOW_TOL       5
MINIMUM_STEP       0.5
THREADS            12
```

[EVAPORATION]

;;Data Source Parameters

```
;;-----
MONTHLY      0.10  0.38  1.12  2.40  3.61  4.57  4.99  4.00  2.24  0.99
0.27  0.07
DRY_ONLY     NO
```

[TEMPERATURE]

```
TIMESERIES    YYC_Temp
WINDSPEED     MONTHLY    14.8 14.6 15. 16.5 16.6 15.6 14 13.2 14.1 14.6 13.7 14.9
SNOWMELT      0 0.5 0.6 1000 51 0.0
ADC           IMPERVIOUS 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0
ADC           PERVIOUS   0.1 0.35 0.53 0.66 0.75 0.82 0.87 0.92 0.95 0.98
```

[RAINGAGES]

```
;;Name      Format      Interval SCF      Source
;;-----
Calgary_24h_100y INTENSITY 0:05      1.0      TIMESERIES Calgary_24h_100y
Calgary_Cont_1960-2014 INTENSITY 1:00      1.0      TIMESERIES YYC_Precipitation
IDE_SC-3_Rainfall INTENSITY 1:00      1.0      FILE      "C:\0-
AM_Files\Projects\Kineticor Final\IDE tool_SC-3_irr.dat" 1001093      MM
```

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Single Events 1:100

[SUBCATCHMENTS]

;;Name	Rain Gage	Outlet	Area	%Imperv	Width	%Slope
CurbLen	SnowPack					
SC-1	Calgary_24h_100y	STG-DEVELOPMENT	325.5	75.5	32550	3
0	Snowpack1					
SC-2	Calgary_24h_100y	STG-DEVELOPMENT	47.8	75.5	4780	3
0	Snowpack1					
SC-3	Calgary_24h_100y	OF-N-1	71.7	0	1434	4
0	Snowpack2					
SC-POND	Calgary_24h_100y	STG-POND	8.3	86.75	1660	20
0	Snowpack2					

[SUBAREAS]

;;Subcatchment	N-Imperv	N-Perv	S-Imperv	S-Perv	PctZero	RouteTo
PctRouted						
SC-1	0.015	0.25	1.6	3.2	0	PERVIOUS 30
SC-2	0.015	0.25	1.6	3.2	0	PERVIOUS 30
SC-3	0.015	0.25	1.6	5	0	PERVIOUS
100						
SC-POND	0.015	0.25	1.6	3.2	100	OUTLET

[INFILTRATION]

;;Subcatchment	Param1	Param2	Param3	Param4	Param5
SC-1	75	7.5	4.14	7	0
SC-2	75	7.5	4.14	7	0
SC-3	75	7.5	4.14	7	0
SC-POND	75	7.5	4.14	7	0

[SNOWPACKS]

;;Name	Surface	Parameters
Snowpack1	PLOWABLE	0.05
0.00		0.2
0.3		0.0
Snowpack1	IMPERVIOUS	0.05
0.00		0.2
25		0.0
Snowpack1	PERVIOUS	0.05
0.00		0.2
100		0.0
Snowpack1	REMOVAL	25
0.0		0.0
Snowpack2	PLOWABLE	0.05
0.00		0.2
0.0		0.0
Snowpack2	IMPERVIOUS	0.05
0.00		0.2
25		0.0
Snowpack2	PERVIOUS	0.05
0.00		0.2
100		0.0
Snowpack2	REMOVAL	25
0.0		0.0

[OUTFALLS]

;;Name	Elevation	Type	Stage Data	Gated	Route To
OF-IRRIGATION	1025	FREE		NO	
OF-N-1	1018	FREE		NO	
OF-N-2	1022.5	FREE		NO	
OF-S-1	1033	FREE		NO	

[STORAGE]

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Single Events 1:100

```

;;Name      Elev.      MaxDepth      InitDepth      Shape      Curve Name/Params
SurDepth Fevap      Psi      Ksat      IMD
;;-----
STG-DEVELOPMENT  1025      0.3      0      FUNCTIONAL  280000      0      0
0      1
STG-POND      1019.5      5.5      2.75      TABULAR      Curve-STG-POND-5.5
0      1

[PUMPS]
;;Name      From Node      To Node      Pump Curve      Status      Startup
Shutoff
;;-----
P-IRRIGATION  STG-POND      OF-IRRIGATION  Curve-irrigation-100  OFF      2.01
2
P-POND-OUT    STG-POND      OF-S-1      Curve-discharge  OFF      3.51
3

[ORIFICES]
;;Name      From Node      To Node      Type      Offset      Qcoeff
Gated      CloseTime
;;-----
OR-POND-OUT  STG-POND      OF-N-2      SIDE      1023      0.65
YES      0

[OUTLETS]
;;Name      From Node      To Node      Offset      Type
QTable/Qcoeff      Qexpon      Gated
;;-----
OL-POND-IN  STG-DEVELOPMENT  STG-POND      1025      TABULAR/DEPTH      Curve-
OL-IN-90-1/s/ha      YES

[XSECTIONS]
;;Link      Shape      Geom1      Geom2      Geom3      Geom4
Barrels      Culvert
;;-----
OR-POND-OUT  RECT_CLOSED  0.4      1      0      0

[CONTROLS]
RULE 1
IF SIMULATION MONTH >= 5
AND SIMULATION MONTH <= 9
AND SIMULATION CLOCKTIME >= 00:00:00
AND SIMULATION CLOCKTIME < 05:00:00
THEN PUMP P-IRRIGATION STATUS = ON
ELSE PUMP P-IRRIGATION STATUS = OFF

[CURVES]
;;Name      Type      X-Value      Y-Value
;;-----
Curve-discharge  Pump2      0      0
Curve-discharge      3.5      0
Curve-discharge      3.51      0.55
Curve-discharge      4      0.55
Curve-discharge      4.01      1.1
Curve-discharge      4.5      1.1
Curve-discharge      4.51      1.65
Curve-discharge      5      1.65
Curve-discharge      5.01      2.2

```

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Single Events 1:100

Curve-discharge	5.5	2.2
Curve-irrigation-100 Pump2	0	0
Curve-irrigation-100	5.5	0
Curve-Irrigation-Updated Pump2	0	0
Curve-Irrigation-Updated	2	0
Curve-Irrigation-Updated	2.01	0.245
Curve-Irrigation-Updated	4	0.245
Curve-Irrigation-Updated	4.01	0
Curve-Irrigation-Updated	5.5	0
Curve-Muni-Backup Rating	0	5
Curve-Muni-Backup	5	5
Curve-OL-IN-90-1/s/ha Rating	0	0
Curve-OL-IN-90-1/s/ha	0.2	33.6
Curve-OL-IN-90-1/s/ha	0.3	33.6
Curve-STG-POND Storage	0	45500
Curve-STG-POND	5	72000
Curve-STG-POND-5.5 Storage	0	43125
Curve-STG-POND-5.5	1	47925
Curve-STG-POND-5.5	2	52925
Curve-STG-POND-5.5	2.5	55500
Curve-STG-POND-5.5	3.5	60800
Curve-STG-POND-5.5	4.5	66300
Curve-STG-POND-5.5	5.5	72000
Curve-STG-POND-5.5	6	74925
Curve-STG-POND-6 Storage	0	40800
Curve-STG-POND-6	1	45500
Curve-STG-POND-6	6	72000

[TIMESERIES]

```
;;Name      Date      Time      Value
;;-----
;Calgary_24h_100y design storm, rain interval = 5 minutes, rain units = mm/hr.
Calgary_24h_100y      0:00      0
Calgary_24h_100y      0:05      1.094
Calgary_24h_100y      0:10      1.103
Calgary_24h_100y      0:15      1.113
Calgary_24h_100y      0:20      1.122
Calgary_24h_100y      0:25      1.132
Calgary_24h_100y      0:30      1.143
Calgary_24h_100y      0:35      1.153
Calgary_24h_100y      0:40      1.163
Calgary_24h_100y      0:45      1.174
Calgary_24h_100y      0:50      1.185
Calgary_24h_100y      0:55      1.197
Calgary_24h_100y      1:00      1.208
Calgary_24h_100y      1:05      1.22
Calgary_24h_100y      1:10      1.232
Calgary_24h_100y      1:15      1.245
Calgary_24h_100y      1:20      1.257
Calgary_24h_100y      1:25      1.27
Calgary_24h_100y      1:30      1.284
Calgary_24h_100y      1:35      1.297
Calgary_24h_100y      1:40      1.311
Calgary_24h_100y      1:45      1.326
Calgary_24h_100y      1:50      1.341
Calgary_24h_100y      1:55      1.356
```

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Single Events 1:100

Calgary_24h_100y	2:00	1.372
Calgary_24h_100y	2:05	1.388
Calgary_24h_100y	2:10	1.404
Calgary_24h_100y	2:15	1.421
Calgary_24h_100y	2:20	1.439
Calgary_24h_100y	2:25	1.457
Calgary_24h_100y	2:30	1.476
Calgary_24h_100y	2:35	1.495
Calgary_24h_100y	2:40	1.515
Calgary_24h_100y	2:45	1.535
Calgary_24h_100y	2:50	1.556
Calgary_24h_100y	2:55	1.578
Calgary_24h_100y	3:00	1.601
Calgary_24h_100y	3:05	1.624
Calgary_24h_100y	3:10	1.648
Calgary_24h_100y	3:15	1.674
Calgary_24h_100y	3:20	1.7
Calgary_24h_100y	3:25	1.727
Calgary_24h_100y	3:30	1.755
Calgary_24h_100y	3:35	1.784
Calgary_24h_100y	3:40	1.815
Calgary_24h_100y	3:45	1.846
Calgary_24h_100y	3:50	1.88
Calgary_24h_100y	3:55	1.914
Calgary_24h_100y	4:00	1.95
Calgary_24h_100y	4:05	1.988
Calgary_24h_100y	4:10	2.028
Calgary_24h_100y	4:15	2.07
Calgary_24h_100y	4:20	2.113
Calgary_24h_100y	4:25	2.159
Calgary_24h_100y	4:30	2.208
Calgary_24h_100y	4:35	2.259
Calgary_24h_100y	4:40	2.313
Calgary_24h_100y	4:45	2.371
Calgary_24h_100y	4:50	2.432
Calgary_24h_100y	4:55	2.497
Calgary_24h_100y	5:00	2.566
Calgary_24h_100y	5:05	2.64
Calgary_24h_100y	5:10	2.719
Calgary_24h_100y	5:15	2.805
Calgary_24h_100y	5:20	2.897
Calgary_24h_100y	5:25	2.997
Calgary_24h_100y	5:30	3.105
Calgary_24h_100y	5:35	3.224
Calgary_24h_100y	5:40	3.354
Calgary_24h_100y	5:45	3.497
Calgary_24h_100y	5:50	3.656
Calgary_24h_100y	5:55	3.833
Calgary_24h_100y	6:00	4.033
Calgary_24h_100y	6:05	4.259
Calgary_24h_100y	6:10	4.519
Calgary_24h_100y	6:15	4.821
Calgary_24h_100y	6:20	5.176
Calgary_24h_100y	6:25	5.601
Calgary_24h_100y	6:30	6.12
Calgary_24h_100y	6:35	6.773
Calgary_24h_100y	6:40	7.624
Calgary_24h_100y	6:45	8.785
Calgary_24h_100y	6:50	10.488
Calgary_24h_100y	6:55	13.283
Calgary_24h_100y	7:00	18.961
Calgary_24h_100y	7:05	40.516
Calgary_24h_100y	7:10	168.138

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Single Events 1:100

Calgary_24h_100y	7:15	54.372
Calgary_24h_100y	7:20	31.748
Calgary_24h_100y	7:25	23.236
Calgary_24h_100y	7:30	18.66
Calgary_24h_100y	7:35	15.763
Calgary_24h_100y	7:40	13.746
Calgary_24h_100y	7:45	12.251
Calgary_24h_100y	7:50	11.093
Calgary_24h_100y	7:55	10.166
Calgary_24h_100y	8:00	9.405
Calgary_24h_100y	8:05	8.768
Calgary_24h_100y	8:10	8.225
Calgary_24h_100y	8:15	7.756
Calgary_24h_100y	8:20	7.346
Calgary_24h_100y	8:25	6.985
Calgary_24h_100y	8:30	6.664
Calgary_24h_100y	8:35	6.376
Calgary_24h_100y	8:40	6.116
Calgary_24h_100y	8:45	5.88
Calgary_24h_100y	8:50	5.665
Calgary_24h_100y	8:55	5.468
Calgary_24h_100y	9:00	5.287
Calgary_24h_100y	9:05	5.119
Calgary_24h_100y	9:10	4.964
Calgary_24h_100y	9:15	4.819
Calgary_24h_100y	9:20	4.684
Calgary_24h_100y	9:25	4.558
Calgary_24h_100y	9:30	4.44
Calgary_24h_100y	9:35	4.329
Calgary_24h_100y	9:40	4.224
Calgary_24h_100y	9:45	4.125
Calgary_24h_100y	9:50	4.032
Calgary_24h_100y	9:55	3.943
Calgary_24h_100y	10:00	3.859
Calgary_24h_100y	10:05	3.78
Calgary_24h_100y	10:10	3.704
Calgary_24h_100y	10:15	3.631
Calgary_24h_100y	10:20	3.562
Calgary_24h_100y	10:25	3.496
Calgary_24h_100y	10:30	3.433
Calgary_24h_100y	10:35	3.373
Calgary_24h_100y	10:40	3.315
Calgary_24h_100y	10:45	3.259
Calgary_24h_100y	10:50	3.206
Calgary_24h_100y	10:55	3.154
Calgary_24h_100y	11:00	3.105
Calgary_24h_100y	11:05	3.057
Calgary_24h_100y	11:10	3.011
Calgary_24h_100y	11:15	2.967
Calgary_24h_100y	11:20	2.924
Calgary_24h_100y	11:25	2.883
Calgary_24h_100y	11:30	2.843
Calgary_24h_100y	11:35	2.805
Calgary_24h_100y	11:40	2.767
Calgary_24h_100y	11:45	2.731
Calgary_24h_100y	11:50	2.696
Calgary_24h_100y	11:55	2.662
Calgary_24h_100y	12:00	2.629
Calgary_24h_100y	12:05	2.597
Calgary_24h_100y	12:10	2.566
Calgary_24h_100y	12:15	2.536
Calgary_24h_100y	12:20	2.506
Calgary_24h_100y	12:25	2.478

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Single Events 1:100

Calgary_24h_100y	12:30	2.45
Calgary_24h_100y	12:35	2.423
Calgary_24h_100y	12:40	2.396
Calgary_24h_100y	12:45	2.371
Calgary_24h_100y	12:50	2.346
Calgary_24h_100y	12:55	2.321
Calgary_24h_100y	13:00	2.297
Calgary_24h_100y	13:05	2.274
Calgary_24h_100y	13:10	2.252
Calgary_24h_100y	13:15	2.229
Calgary_24h_100y	13:20	2.208
Calgary_24h_100y	13:25	2.187
Calgary_24h_100y	13:30	2.166
Calgary_24h_100y	13:35	2.146
Calgary_24h_100y	13:40	2.126
Calgary_24h_100y	13:45	2.107
Calgary_24h_100y	13:50	2.088
Calgary_24h_100y	13:55	2.069
Calgary_24h_100y	14:00	2.051
Calgary_24h_100y	14:05	2.034
Calgary_24h_100y	14:10	2.016
Calgary_24h_100y	14:15	1.999
Calgary_24h_100y	14:20	1.983
Calgary_24h_100y	14:25	1.966
Calgary_24h_100y	14:30	1.95
Calgary_24h_100y	14:35	1.935
Calgary_24h_100y	14:40	1.919
Calgary_24h_100y	14:45	1.904
Calgary_24h_100y	14:50	1.889
Calgary_24h_100y	14:55	1.875
Calgary_24h_100y	15:00	1.86
Calgary_24h_100y	15:05	1.846
Calgary_24h_100y	15:10	1.833
Calgary_24h_100y	15:15	1.819
Calgary_24h_100y	15:20	1.806
Calgary_24h_100y	15:25	1.793
Calgary_24h_100y	15:30	1.78
Calgary_24h_100y	15:35	1.767
Calgary_24h_100y	15:40	1.755
Calgary_24h_100y	15:45	1.743
Calgary_24h_100y	15:50	1.731
Calgary_24h_100y	15:55	1.719
Calgary_24h_100y	16:00	1.707
Calgary_24h_100y	16:05	1.696
Calgary_24h_100y	16:10	1.685
Calgary_24h_100y	16:15	1.673
Calgary_24h_100y	16:20	1.663
Calgary_24h_100y	16:25	1.652
Calgary_24h_100y	16:30	1.641
Calgary_24h_100y	16:35	1.631
Calgary_24h_100y	16:40	1.621
Calgary_24h_100y	16:45	1.611
Calgary_24h_100y	16:50	1.601
Calgary_24h_100y	16:55	1.591
Calgary_24h_100y	17:00	1.581
Calgary_24h_100y	17:05	1.572
Calgary_24h_100y	17:10	1.562
Calgary_24h_100y	17:15	1.553
Calgary_24h_100y	17:20	1.544
Calgary_24h_100y	17:25	1.535
Calgary_24h_100y	17:30	1.526
Calgary_24h_100y	17:35	1.517
Calgary_24h_100y	17:40	1.509

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Single Events 1:100

Calgary_24h_100y	17:45	1.5
Calgary_24h_100y	17:50	1.492
Calgary_24h_100y	17:55	1.484
Calgary_24h_100y	18:00	1.476
Calgary_24h_100y	18:05	1.467
Calgary_24h_100y	18:10	1.46
Calgary_24h_100y	18:15	1.452
Calgary_24h_100y	18:20	1.444
Calgary_24h_100y	18:25	1.436
Calgary_24h_100y	18:30	1.429
Calgary_24h_100y	18:35	1.421
Calgary_24h_100y	18:40	1.414
Calgary_24h_100y	18:45	1.407
Calgary_24h_100y	18:50	1.399
Calgary_24h_100y	18:55	1.392
Calgary_24h_100y	19:00	1.385
Calgary_24h_100y	19:05	1.378
Calgary_24h_100y	19:10	1.372
Calgary_24h_100y	19:15	1.365
Calgary_24h_100y	19:20	1.358
Calgary_24h_100y	19:25	1.352
Calgary_24h_100y	19:30	1.345
Calgary_24h_100y	19:35	1.339
Calgary_24h_100y	19:40	1.332
Calgary_24h_100y	19:45	1.326
Calgary_24h_100y	19:50	1.32
Calgary_24h_100y	19:55	1.313
Calgary_24h_100y	20:00	1.307
Calgary_24h_100y	20:05	1.301
Calgary_24h_100y	20:10	1.295
Calgary_24h_100y	20:15	1.289
Calgary_24h_100y	20:20	1.284
Calgary_24h_100y	20:25	1.278
Calgary_24h_100y	20:30	1.272
Calgary_24h_100y	20:35	1.266
Calgary_24h_100y	20:40	1.261
Calgary_24h_100y	20:45	1.255
Calgary_24h_100y	20:50	1.25
Calgary_24h_100y	20:55	1.244
Calgary_24h_100y	21:00	1.239
Calgary_24h_100y	21:05	1.234
Calgary_24h_100y	21:10	1.229
Calgary_24h_100y	21:15	1.223
Calgary_24h_100y	21:20	1.218
Calgary_24h_100y	21:25	1.213
Calgary_24h_100y	21:30	1.208
Calgary_24h_100y	21:35	1.203
Calgary_24h_100y	21:40	1.198
Calgary_24h_100y	21:45	1.193
Calgary_24h_100y	21:50	1.188
Calgary_24h_100y	21:55	1.184
Calgary_24h_100y	22:00	1.179
Calgary_24h_100y	22:05	1.174
Calgary_24h_100y	22:10	1.17
Calgary_24h_100y	22:15	1.165
Calgary_24h_100y	22:20	1.16
Calgary_24h_100y	22:25	1.156
Calgary_24h_100y	22:30	1.151
Calgary_24h_100y	22:35	1.147
Calgary_24h_100y	22:40	1.143
Calgary_24h_100y	22:45	1.138
Calgary_24h_100y	22:50	1.134
Calgary_24h_100y	22:55	1.13

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Single Events 1:100

Calgary_24h_100y	23:00	1.125
Calgary_24h_100y	23:05	1.121
Calgary_24h_100y	23:10	1.117
Calgary_24h_100y	23:15	1.113
Calgary_24h_100y	23:20	1.109
Calgary_24h_100y	23:25	1.105
Calgary_24h_100y	23:30	1.101
Calgary_24h_100y	23:35	1.097
Calgary_24h_100y	23:40	1.093
Calgary_24h_100y	23:45	1.089
Calgary_24h_100y	23:50	1.085
Calgary_24h_100y	23:55	1.081
Calgary_24h_100y	24:00	1.077

IDE-Withdrawal FILE "C:\0-AM_Files\Projects\Kineticor Final\IDE
tool_PondWithdrawal.txt"

YYC_Precipitation FILE "C:\Users\Ajay.Muthukumar\Cima+\Z0026600-Kineticor-Hwy 566
North Calgary Site - Documents_Documents\300_CONC_DES\399_Stormwater\PCSWMM\Precip
1960-2014.dat"

YYC_Temp FILE "C:\Users\Ajay.Muthukumar\Cima+\Z0026600-Kineticor-Hwy 566
North Calgary Site -
Documents_Documents\300_CONC_DES\399_Stormwater\PCSWMM\CalgTempHrly1960_2014.dat"

[REPORT]

;;Reporting Options
INPUT YES
CONTROLS NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL

[ADJUSTMENTS]

;;Parameter	Subcatchment	Monthly Adjustments								
CONDUCTIVITY			0.05	0.05	0.05	0.05	1.0	1.0	1.0	1.0
1.0	1.0	0.05	0.05							

[TAGS]

Subcatch	SC-1	Developed
Subcatch	SC-2	Developed
Subcatch	SC-3	Undeveloped
Subcatch	SC-POND	Pond

[MAP]

DIMENSIONS	11723.78165	5675083.43005	14425.01535	5677730.90895
UNITS	Meters			

[COORDINATES]

;;Node	X-Coord	Y-Coord
OF-IRRIGATION	14093.209	5676880.175
OF-N-1	14287.972	5677069.091
OF-N-2	14285.091	5677021.142
OF-S-1	14302.232	5675813.051
STG-DEVELOPMENT	13801.186	5676473.424
STG-POND	14164.545	5676480.257

[VERTICES]

;;Link	X-Coord	Y-Coord
;;-----	-----	-----

[POLYGONS]

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Single Events 1:100

;;Subcatchment	X-Coord	Y-Coord
;;-----	-----	-----
SC-1	13498.632	5677046.219
SC-1	12647.703	5677046.341
SC-1	12650.248	5676805.025
SC-1	11846.565	5676798.811
SC-1	11849.474	5675994.149
SC-1	12652.92	5675999.973
SC-1	12655.809	5675205.662
SC-1	14270.572	5675203.77
SC-1	14270.572	5676253.596
SC-1	14069.867	5676254.433
SC-1	14070.993	5676671.22
SC-1	13500.068	5676672.762
SC-1	13498.632	5677046.219
SC-2	12647.703	5677046.341
SC-2	12646.12	5677610.569
SC-2	13496.342	5677605.354
SC-2	13498.632	5677046.219
SC-2	12647.703	5677046.341
SC-3	13500.068	5676672.762
SC-3	13496.342	5677605.354
SC-3	14265.915	5677601.944
SC-3	14269.307	5676670.684
SC-3	13500.068	5676672.762
SC-POND	14071.7	5676671.927
SC-POND	14269.307	5676670.684
SC-POND	14270.572	5676253.596
SC-POND	14069.867	5676254.433
SC-POND	14071.7	5676671.927

;;Storage Node	X-Coord	Y-Coord
;;-----	-----	-----

[SYMBOLS]

;;Gage	X-Coord	Y-Coord
;;-----	-----	-----

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.2 (Build 5.2.4)

Element Count

Number of rain gages 3
Number of subcatchments ... 4
Number of nodes 6
Number of links 4
Number of pollutants 0
Number of land uses 0

Rainage Summary

Name	Data Source	Data Type	Recording Interval
Calgary_24h_100y	Calgary_24h_100y	INTENSITY	5 min.
Calgary_Cont_1960-2014	YYC_Precipitation	INTENSITY	60 min.
IDE_SC-3_Rainfall	C:\0-AM_Files\Projects\Kineticor Final\IDE tool_SC-3_irr.dat		

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage
Outlet					
SC-1	325.50	32550.00	75.50	3.0000	Calgary_24h_100y
STG-DEVELOPMENT					
SC-2	47.80	4780.00	75.50	3.0000	Calgary_24h_100y
STG-DEVELOPMENT					
SC-3	71.70	1434.00	0.00	4.0000	Calgary_24h_100y
OF-N-1					
SC-POND	8.30	1660.00	86.75	20.0000	Calgary_24h_100y
STG-POND					

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
OF-IRRIGATION	OUTFALL	1025.00	0.00	0.0	
OF-N-1	OUTFALL	1018.00	0.00	0.0	
OF-N-2	OUTFALL	1022.50	0.00	0.0	
OF-S-1	OUTFALL	1033.00	0.00	0.0	
STG-DEVELOPMENT	STORAGE	1025.00	0.30	0.0	
STG-POND	STORAGE	1019.50	5.50	0.0	

Link Summary

Master Drainage Plan

Kineticor ASP

PCSWMM Output File

Single Event Analysis 1:100

Name	From Node	To Node	Type	Length	%Slope
Roughness					
P-IRRIGATION	STG-POND	OF-IRRIGATION	TYPE2 PUMP		
P-POND-OUT	STG-POND	OF-S-1	TYPE2 PUMP		
OR-POND-OUT	STG-POND	OF-N-2	ORIFICE		
OL-POND-IN	STG-DEVELOPMENT	STG-POND	OUTLET		

Cross Section Summary

	Full	Full	Hyd.	Max.	No. of
Conduit	Shape	Depth	Area	Rad.	Width Barrels
Flow					

Analysis Options

Flow Units CMS

Process Models:

Rainfall/Runoff YES

RDII NO

Snowmelt YES

Groundwater NO

Flow Routing YES

Ponding Allowed NO

Water Quality NO

Infiltration Method HORTON

Flow Routing Method DYNWAVE

Surcharge Method EXTRAN

Starting Date 07/01/2014 00:00:00

Ending Date 07/07/2014 23:00:00

Antecedent Dry Days 0.0

Report Time Step 00:05:00

Wet Time Step 00:00:05

Dry Time Step 00:00:05

Routing Time Step 5.00 sec

Variable Time Step YES

Maximum Trials 8

Number of Threads 1

Head Tolerance 0.001500 m

Rainfall File Summary

Station ID	First Date	Last Date	Recording Frequency	Periods w/Precip	Periods Missing	Periods Malfunc.
1001093	01/02/1960	12/29/2014	60 min	45403	0	0

Runoff Quantity Continuity

Volume

hectare-m

Depth

mm

Master Drainage Plan

Kineticor ASP

PCSWMM Output File

Single Event Analysis 1:100

Initial Snow Cover	0.000	0.000
Total Precipitation	40.646	89.667
Evaporation Loss	2.127	4.693
Infiltration Loss	14.705	32.440
Surface Runoff	23.814	52.535
Snow Removed	0.000	0.000
Final Snow Cover	0.000	0.000
Final Storage	0.000	0.000
Continuity Error (%)	-0.000	

	Volume hectare-m	Volume 10^6 ltr
*****	-----	-----
Flow Routing Continuity		

Dry Weather Inflow	0.000	0.000
Wet Weather Inflow	23.814	238.142
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	19.216	192.160
Flooding Loss	0.000	0.000
Evaporation Loss	0.353	3.535
Exfiltration Loss	0.000	0.000
Initial Stored Volume	13.708	137.083
Final Stored Volume	17.952	179.525
Continuity Error (%)	0.001	

Time-Step Critical Elements

None

Highest Flow Instability Indexes

All links are stable.

Most Frequent Nonconverging Nodes

Convergence obtained at all time steps.

Routing Time Step Summary

Minimum Time Step	:	4.50 sec
Average Time Step	:	5.00 sec
Maximum Time Step	:	5.00 sec
% of Time in Steady State	:	0.00
Average Iterations per Step	:	2.00
% of Steps Not Converging	:	0.00
Time Step Frequencies	:	
5.000 - 3.155 sec	:	100.00 %
3.155 - 1.991 sec	:	0.00 %
1.991 - 1.256 sec	:	0.00 %
1.256 - 0.792 sec	:	0.00 %
0.792 - 0.500 sec	:	0.00 %

Subcatchment Runoff Summary

Perv	Total	Total	Total	Total	Total	Total	Imperv
Runoff	Runoff	Precip	Peak	Runoff	Evap	Infil	Runoff
Subcatchment	Runoff	Runoff	Runoff	Coeff	mm	mm	mm
mm	mm	10^6 ltr	mm	mm	mm	mm	mm
			CMS				
SC-1		89.67		0.00	5.39	24.29	62.60
16.17	59.99	195.27	111.46	0.669			
SC-2		89.67		0.00	5.39	24.29	62.60
16.17	59.99	28.67	16.37	0.669			
SC-3		89.67		0.00	1.08	77.61	0.00
10.97	10.97	7.87	1.22	0.122			
SC-POND		89.67		0.00	4.46	8.91	73.40
2.90	76.30	6.33	3.81	0.851			

Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
OF-IRRIGATION	OUTFALL	0.00	0.00	1025.00	0 00:00	0.00
OF-N-1	OUTFALL	0.00	0.00	1018.00	0 00:00	0.00
OF-N-2	OUTFALL	0.00	0.00	1022.50	0 00:00	0.00
OF-S-1	OUTFALL	0.00	0.00	1033.00	0 00:00	0.00
STG-DEVELOPMENT	STORAGE	0.00	0.21	1025.21	0 07:28	0.21
STG-POND	STORAGE	3.58	5.12	1024.62	0 09:55	5.12

Node Inflow Summary

Total	Flow	Maximum	Maximum	Lateral
Inflow	Balance	Lateral	Total	Time of Max
Volume	Error	Inflow	Inflow	Occurrence
Node	Type	CMS	CMS	days hr:min
ltr	Percent			
OF-IRRIGATION	OUTFALL	0.000	0.000	0 00:00
0	0.000 ltr			
OF-N-1	OUTFALL	1.221	1.221	0 07:40
7.87	0.000			

Master Drainage Plan

Kineticor ASP

PCSWMM Output File

Single Event Analysis 1:100

OF-N-2	OUTFALL	0.000	1.366	0	09:55	0
72.5	0.000					
OF-S-1	OUTFALL	0.000	2.200	0	08:49	0
112	0.000					
STG-DEVELOPMENT	STORAGE	127.823	127.823	0	07:15	224
224	-0.000					
STG-POND	STORAGE	3.811	34.378	0	07:22	6.33
366	0.001					

Node Surcharge Summary

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Max	Maximum	Average	Avg	Evap	Exfil	Maximum	Max	Time of
Occurrence	Outflow	Volume	Pcnt	Pcnt	Pcnt	Volume	Pcnt	days
Storage Unit	1000 m³	Full	Loss	Loss	1000 m³	Full		
hr:min	CMS							
STG-DEVELOPMENT	0.620	0.7	0.6	0.0	59.245	70.5	0	
07:28	33.616							
STG-POND	186.722	59.5	0.6	0.0	286.849	91.4	0	
09:55	3.570							

Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow CMS	Max Flow CMS	Total Volume 10^6 ltr
OF-IRRIGATION	0.00	0.000	0.000	0.000
OF-N-1	2.40	0.545	1.221	7.868
OF-N-2	16.75	0.720	1.366	72.469
OF-S-1	16.76	1.110	2.200	111.822
System	8.98	2.375	4.190	192.159

Master Drainage Plan

Kineticor ASP

PCSWMM Output File

Single Event Analysis 1:100

Link Flow Summary

Link	Type	Maximum Flow CMS	Time of Max Occurrence days hr:min	Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
P-IRRIGATION	PUMP	0.000	0 00:00			
P-POND-OUT	PUMP	2.200	0 08:49		1.00	
OR-POND-OUT	ORIFICE	1.366	0 09:55			1.00
OL-POND-IN	DUMMY	33.600	0 07:22			

Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class							
		Up Dry	Down Dry	Sub Crit	Sup Crit	Up Crit	Down Crit	Norm Ltd	Inlet Ctrl

Conduit Surcharge Summary

No conduits were surcharged.

Pumping Summary

Power	% Time Off	Percent	Number of	Min Flow CMS	Avg Flow CMS	Max Flow CMS	Total Volume 10^6 ltr
Usage Pump Kw-hr	Pump Curve Low High	Utilized	Start-Ups				
P-IRRIGATION	0.00 0.0 0.0	0.00	0	0.00	0.00	0.00	0.000
P-POND-OUT	2744.52 0.0 0.0	16.76	1	0.00	1.11	2.20	111.822

Analysis begun on: Mon Jul 21 20:59:59 2025

Analysis ended on: Mon Jul 21 21:00:01 2025

Total elapsed time: 00:00:02

[TITLE]

;;Project Title/Notes

[OPTIONS]

```
;;Option      Value
FLOW_UNITS    CMS
INFILTRATION  HORTON
FLOW_ROUTING  DYNWAVE
LINK_OFFSETS  ELEVATION
MIN_SLOPE     0
ALLOW_PONDING NO
SKIP_STEADY_STATE NO
```

```
START_DATE    01/01/1960
START_TIME    00:00:00
REPORT_START_DATE 01/01/1960
REPORT_START_TIME 00:00:00
END_DATE      12/31/2014
END_TIME      23:00:00
SWEEP_START   01/01
SWEEP_END     12/31
DRY_DAYS      0
REPORT_STEP   01:00:00
WET_STEP      00:15:00
DRY_STEP      00:15:00
ROUTING_STEP  60
RULE_STEP     00:00:00
```

```
INERTIAL_DAMPING  PARTIAL
NORMAL_FLOW_LIMITED BOTH
FORCE_MAIN_EQUATION H-W
VARIABLE_STEP    0.75
LENGTHENING_STEP 0
MIN_SURFAREA     0
MAX_TRIALS       8
HEAD_TOLERANCE   0.0015
SYS_FLOW_TOL     5
LAT_FLOW_TOL     5
MINIMUM_STEP     0.5
THREADS          12
```

[EVAPORATION]

;;Data Source Parameters

```
;;-----
MONTHLY 0.10 0.38 1.12 2.40 3.61 4.57 4.99 4.00 2.24 0.99
0.27 0.07
DRY_ONLY NO
```

[TEMPERATURE]

```
TIMESERIES YYC_Temp
WINDSPEED MONTHLY 14.8 14.6 15. 16.5 16.6 15.6 14 13.2 14.1 14.6 13.7 14.9
SNOWMELT 0 0.5 0.6 1000 51 0.0
ADC IMPERVIOUS 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0 1.0
ADC PERVIOUS 0.1 0.35 0.53 0.66 0.75 0.82 0.87 0.92 0.95 0.98
```

[RAINGAGES]

```
;;Name      Format      Interval SCF      Source
;;-----
Calgary_24h_100y INTENSITY 0:05      1.0      TIMESERIES Calgary_24h_100y
Calgary_Cont_1960-2014 INTENSITY 1:00      1.0      TIMESERIES YYC_Precipitation
IDE_SC-3_Rainfall INTENSITY 1:00      1.0      FILE      "C:\0-
AM_Files\Projects\Kineticor Final\IDE tool_SC-3_irr.dat" 1001093      MM
```

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Continuous Simulation

[SUBCATCHMENTS]

;;Name	Rain Gage	Outlet	Area	%Imperv	Width	%Slope
CurbLen	SnowPack					
SC-1	Calgary_Cont_1960-2014	STG-DEVELOPMENT	325.5	75.5	32550	3
0	Snowpack1					
SC-2	Calgary_Cont_1960-2014	STG-DEVELOPMENT	47.8	75.5	4780	3
0	Snowpack1					
SC-3	Calgary_Cont_1960-2014	OF-N-1	71.7	0	1434	4
0	Snowpack2					
SC-POND	Calgary_Cont_1960-2014	STG-POND	8.3	86.75	1660	20
0	Snowpack2					

[SUBAREAS]

;;Subcatchment	N-Imperv	N-Perv	S-Imperv	S-Perv	PctZero	RouteTo
PctRouted						
SC-1	0.015	0.25	1.6	3.2	0	PERVIOUS 30
SC-2	0.015	0.25	1.6	3.2	0	PERVIOUS 30
SC-3	0.015	0.25	1.6	5	0	PERVIOUS
100						
SC-POND	0.015	0.25	1.6	3.2	100	OUTLET

[INFILTRATION]

;;Subcatchment	Param1	Param2	Param3	Param4	Param5
SC-1	75	7.5	4.14	7	0
SC-2	75	7.5	4.14	7	0
SC-3	75	7.5	4.14	7	0
SC-POND	75	7.5	4.14	7	0

[SNOWPACKS]

;;Name	Surface	Parameters
Snowpack1	PLOWABLE	0.05
0.00		0.2
0.3		0.0
Snowpack1	IMPERVIOUS	0.05
0.00		0.2
25		0.0
Snowpack1	PERVIOUS	0.05
0.00		0.2
100		0.0
Snowpack1	REMOVAL	25
0.0		0.0
Snowpack2	PLOWABLE	0.05
0.00		0.2
0.0		0.0
Snowpack2	IMPERVIOUS	0.05
0.00		0.2
25		0.0
Snowpack2	PERVIOUS	0.05
0.00		0.2
100		0.0
Snowpack2	REMOVAL	25
0.0		0.0

[OUTFALLS]

;;Name	Elevation	Type	Stage Data	Gated	Route To
OF-IRRIGATION	1025	FREE		NO	
OF-N-1	1018	FREE		NO	
OF-N-2	1022.5	FREE		NO	
OF-S-1	1033	FREE		NO	

[STORAGE]

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Continuous Simulation

```

;;Name      Elev.      MaxDepth      InitDepth      Shape      Curve Name/Params
SurDepth Fevap      Psi      Ksat      IMD
;;-----
STG-DEVELOPMENT  1025      0.3      0      FUNCTIONAL  280000      0      0
0      1
STG-POND      1019.5      5.5      2.75      TABULAR      Curve-STG-POND-5.5
0      1

[PUMPS]
;;Name      From Node      To Node      Pump Curve      Status      Startup
Shutoff
;;-----
P-IRRIGATION  STG-POND      OF-IRRIGATION  Curve-Irrigation-Updated  OFF  2.01
2
P-POND-OUT    STG-POND      OF-S-1      Curve-discharge  OFF      3.51
3

[ORIFICES]
;;Name      From Node      To Node      Type      Offset      Qcoeff
Gated      CloseTime
;;-----
OR-POND-OUT  STG-POND      OF-N-2      SIDE      1023      0.65
YES      0

[OUTLETS]
;;Name      From Node      To Node      Offset      Type
QTable/Qcoeff      Qexpon      Gated
;;-----
OL-POND-IN  STG-DEVELOPMENT  STG-POND      1025      TABULAR/DEPTH      Curve-
OL-IN-90-1/s/ha      YES

[XSECTIONS]
;;Link      Shape      Geom1      Geom2      Geom3      Geom4
Barrels      Culvert
;;-----
OR-POND-OUT  RECT_CLOSED  0.4      1      0      0

[CONTROLS]
RULE 1
IF SIMULATION MONTH >= 5
AND SIMULATION MONTH <= 9
AND SIMULATION CLOCKTIME >= 00:00:00
AND SIMULATION CLOCKTIME < 04:59:00
THEN PUMP P-IRRIGATION STATUS = ON
ELSE PUMP P-IRRIGATION STATUS = OFF

[CURVES]
;;Name      Type      X-Value      Y-Value
;;-----
Curve-discharge  Pump2      0      0
Curve-discharge      3.5      0
Curve-discharge      3.51      0.55
Curve-discharge      4      0.55
Curve-discharge      4.01      1.1
Curve-discharge      4.5      1.1
Curve-discharge      4.51      1.65
Curve-discharge      5      1.65
Curve-discharge      5.01      2.2

```

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Continuous Simulation

Curve-discharge	5.5	2.2
Curve-irrigation-100 Pump2	0	0
Curve-irrigation-100	6	0
Curve-Irrigation-Updated Pump2	0	0
Curve-Irrigation-Updated	2	0
Curve-Irrigation-Updated	2.01	0.245
Curve-Irrigation-Updated	4	0.245
Curve-Irrigation-Updated	4.01	0
Curve-Irrigation-Updated	5.5	0
Curve-Muni-Backup Rating	0	5
Curve-Muni-Backup	5	5
Curve-OL-IN-90-1/s/ha Rating	0	0
Curve-OL-IN-90-1/s/ha	0.2	33.6
Curve-OL-IN-90-1/s/ha	0.3	33.6
Curve-STG-POND Storage	0	45500
Curve-STG-POND	5	72000
Curve-STG-POND-5.5 Storage	0	43125
Curve-STG-POND-5.5	1	47925
Curve-STG-POND-5.5	2	52925
Curve-STG-POND-5.5	2.5	55500
Curve-STG-POND-5.5	3.5	60800
Curve-STG-POND-5.5	4.5	66300
Curve-STG-POND-5.5	5.5	72000
Curve-STG-POND-5.5	6	74925
Curve-STG-POND-6 Storage	0	40800
Curve-STG-POND-6	1	45500
Curve-STG-POND-6	6	72000

[TIMESERIES]

```
;;Name      Date      Time      Value
;;-----
;Calgary_24h_100y design storm, rain interval = 5 minutes, rain units = mm/hr.
Calgary_24h_100y      0:00      0
Calgary_24h_100y      0:05      1.094
Calgary_24h_100y      0:10      1.103
Calgary_24h_100y      0:15      1.113
Calgary_24h_100y      0:20      1.122
Calgary_24h_100y      0:25      1.132
Calgary_24h_100y      0:30      1.143
Calgary_24h_100y      0:35      1.153
Calgary_24h_100y      0:40      1.163
Calgary_24h_100y      0:45      1.174
Calgary_24h_100y      0:50      1.185
Calgary_24h_100y      0:55      1.197
Calgary_24h_100y      1:00      1.208
Calgary_24h_100y      1:05      1.22
Calgary_24h_100y      1:10      1.232
Calgary_24h_100y      1:15      1.245
Calgary_24h_100y      1:20      1.257
Calgary_24h_100y      1:25      1.27
Calgary_24h_100y      1:30      1.284
Calgary_24h_100y      1:35      1.297
Calgary_24h_100y      1:40      1.311
Calgary_24h_100y      1:45      1.326
Calgary_24h_100y      1:50      1.341
Calgary_24h_100y      1:55      1.356
```

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Continuous Simulation

Calgary_24h_100y	2:00	1.372
Calgary_24h_100y	2:05	1.388
Calgary_24h_100y	2:10	1.404
Calgary_24h_100y	2:15	1.421
Calgary_24h_100y	2:20	1.439
Calgary_24h_100y	2:25	1.457
Calgary_24h_100y	2:30	1.476
Calgary_24h_100y	2:35	1.495
Calgary_24h_100y	2:40	1.515
Calgary_24h_100y	2:45	1.535
Calgary_24h_100y	2:50	1.556
Calgary_24h_100y	2:55	1.578
Calgary_24h_100y	3:00	1.601
Calgary_24h_100y	3:05	1.624
Calgary_24h_100y	3:10	1.648
Calgary_24h_100y	3:15	1.674
Calgary_24h_100y	3:20	1.7
Calgary_24h_100y	3:25	1.727
Calgary_24h_100y	3:30	1.755
Calgary_24h_100y	3:35	1.784
Calgary_24h_100y	3:40	1.815
Calgary_24h_100y	3:45	1.846
Calgary_24h_100y	3:50	1.88
Calgary_24h_100y	3:55	1.914
Calgary_24h_100y	4:00	1.95
Calgary_24h_100y	4:05	1.988
Calgary_24h_100y	4:10	2.028
Calgary_24h_100y	4:15	2.07
Calgary_24h_100y	4:20	2.113
Calgary_24h_100y	4:25	2.159
Calgary_24h_100y	4:30	2.208
Calgary_24h_100y	4:35	2.259
Calgary_24h_100y	4:40	2.313
Calgary_24h_100y	4:45	2.371
Calgary_24h_100y	4:50	2.432
Calgary_24h_100y	4:55	2.497
Calgary_24h_100y	5:00	2.566
Calgary_24h_100y	5:05	2.64
Calgary_24h_100y	5:10	2.719
Calgary_24h_100y	5:15	2.805
Calgary_24h_100y	5:20	2.897
Calgary_24h_100y	5:25	2.997
Calgary_24h_100y	5:30	3.105
Calgary_24h_100y	5:35	3.224
Calgary_24h_100y	5:40	3.354
Calgary_24h_100y	5:45	3.497
Calgary_24h_100y	5:50	3.656
Calgary_24h_100y	5:55	3.833
Calgary_24h_100y	6:00	4.033
Calgary_24h_100y	6:05	4.259
Calgary_24h_100y	6:10	4.519
Calgary_24h_100y	6:15	4.821
Calgary_24h_100y	6:20	5.176
Calgary_24h_100y	6:25	5.601
Calgary_24h_100y	6:30	6.12
Calgary_24h_100y	6:35	6.773
Calgary_24h_100y	6:40	7.624
Calgary_24h_100y	6:45	8.785
Calgary_24h_100y	6:50	10.488
Calgary_24h_100y	6:55	13.283
Calgary_24h_100y	7:00	18.961
Calgary_24h_100y	7:05	40.516
Calgary_24h_100y	7:10	168.138

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Continuous Simulation

Calgary_24h_100y	7:15	54.372
Calgary_24h_100y	7:20	31.748
Calgary_24h_100y	7:25	23.236
Calgary_24h_100y	7:30	18.66
Calgary_24h_100y	7:35	15.763
Calgary_24h_100y	7:40	13.746
Calgary_24h_100y	7:45	12.251
Calgary_24h_100y	7:50	11.093
Calgary_24h_100y	7:55	10.166
Calgary_24h_100y	8:00	9.405
Calgary_24h_100y	8:05	8.768
Calgary_24h_100y	8:10	8.225
Calgary_24h_100y	8:15	7.756
Calgary_24h_100y	8:20	7.346
Calgary_24h_100y	8:25	6.985
Calgary_24h_100y	8:30	6.664
Calgary_24h_100y	8:35	6.376
Calgary_24h_100y	8:40	6.116
Calgary_24h_100y	8:45	5.88
Calgary_24h_100y	8:50	5.665
Calgary_24h_100y	8:55	5.468
Calgary_24h_100y	9:00	5.287
Calgary_24h_100y	9:05	5.119
Calgary_24h_100y	9:10	4.964
Calgary_24h_100y	9:15	4.819
Calgary_24h_100y	9:20	4.684
Calgary_24h_100y	9:25	4.558
Calgary_24h_100y	9:30	4.44
Calgary_24h_100y	9:35	4.329
Calgary_24h_100y	9:40	4.224
Calgary_24h_100y	9:45	4.125
Calgary_24h_100y	9:50	4.032
Calgary_24h_100y	9:55	3.943
Calgary_24h_100y	10:00	3.859
Calgary_24h_100y	10:05	3.78
Calgary_24h_100y	10:10	3.704
Calgary_24h_100y	10:15	3.631
Calgary_24h_100y	10:20	3.562
Calgary_24h_100y	10:25	3.496
Calgary_24h_100y	10:30	3.433
Calgary_24h_100y	10:35	3.373
Calgary_24h_100y	10:40	3.315
Calgary_24h_100y	10:45	3.259
Calgary_24h_100y	10:50	3.206
Calgary_24h_100y	10:55	3.154
Calgary_24h_100y	11:00	3.105
Calgary_24h_100y	11:05	3.057
Calgary_24h_100y	11:10	3.011
Calgary_24h_100y	11:15	2.967
Calgary_24h_100y	11:20	2.924
Calgary_24h_100y	11:25	2.883
Calgary_24h_100y	11:30	2.843
Calgary_24h_100y	11:35	2.805
Calgary_24h_100y	11:40	2.767
Calgary_24h_100y	11:45	2.731
Calgary_24h_100y	11:50	2.696
Calgary_24h_100y	11:55	2.662
Calgary_24h_100y	12:00	2.629
Calgary_24h_100y	12:05	2.597
Calgary_24h_100y	12:10	2.566
Calgary_24h_100y	12:15	2.536
Calgary_24h_100y	12:20	2.506
Calgary_24h_100y	12:25	2.478

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Continuous Simulation

Calgary_24h_100y	12:30	2.45
Calgary_24h_100y	12:35	2.423
Calgary_24h_100y	12:40	2.396
Calgary_24h_100y	12:45	2.371
Calgary_24h_100y	12:50	2.346
Calgary_24h_100y	12:55	2.321
Calgary_24h_100y	13:00	2.297
Calgary_24h_100y	13:05	2.274
Calgary_24h_100y	13:10	2.252
Calgary_24h_100y	13:15	2.229
Calgary_24h_100y	13:20	2.208
Calgary_24h_100y	13:25	2.187
Calgary_24h_100y	13:30	2.166
Calgary_24h_100y	13:35	2.146
Calgary_24h_100y	13:40	2.126
Calgary_24h_100y	13:45	2.107
Calgary_24h_100y	13:50	2.088
Calgary_24h_100y	13:55	2.069
Calgary_24h_100y	14:00	2.051
Calgary_24h_100y	14:05	2.034
Calgary_24h_100y	14:10	2.016
Calgary_24h_100y	14:15	1.999
Calgary_24h_100y	14:20	1.983
Calgary_24h_100y	14:25	1.966
Calgary_24h_100y	14:30	1.95
Calgary_24h_100y	14:35	1.935
Calgary_24h_100y	14:40	1.919
Calgary_24h_100y	14:45	1.904
Calgary_24h_100y	14:50	1.889
Calgary_24h_100y	14:55	1.875
Calgary_24h_100y	15:00	1.86
Calgary_24h_100y	15:05	1.846
Calgary_24h_100y	15:10	1.833
Calgary_24h_100y	15:15	1.819
Calgary_24h_100y	15:20	1.806
Calgary_24h_100y	15:25	1.793
Calgary_24h_100y	15:30	1.78
Calgary_24h_100y	15:35	1.767
Calgary_24h_100y	15:40	1.755
Calgary_24h_100y	15:45	1.743
Calgary_24h_100y	15:50	1.731
Calgary_24h_100y	15:55	1.719
Calgary_24h_100y	16:00	1.707
Calgary_24h_100y	16:05	1.696
Calgary_24h_100y	16:10	1.685
Calgary_24h_100y	16:15	1.673
Calgary_24h_100y	16:20	1.663
Calgary_24h_100y	16:25	1.652
Calgary_24h_100y	16:30	1.641
Calgary_24h_100y	16:35	1.631
Calgary_24h_100y	16:40	1.621
Calgary_24h_100y	16:45	1.611
Calgary_24h_100y	16:50	1.601
Calgary_24h_100y	16:55	1.591
Calgary_24h_100y	17:00	1.581
Calgary_24h_100y	17:05	1.572
Calgary_24h_100y	17:10	1.562
Calgary_24h_100y	17:15	1.553
Calgary_24h_100y	17:20	1.544
Calgary_24h_100y	17:25	1.535
Calgary_24h_100y	17:30	1.526
Calgary_24h_100y	17:35	1.517
Calgary_24h_100y	17:40	1.509

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Continuous Simulation

Calgary_24h_100y	17:45	1.5
Calgary_24h_100y	17:50	1.492
Calgary_24h_100y	17:55	1.484
Calgary_24h_100y	18:00	1.476
Calgary_24h_100y	18:05	1.467
Calgary_24h_100y	18:10	1.46
Calgary_24h_100y	18:15	1.452
Calgary_24h_100y	18:20	1.444
Calgary_24h_100y	18:25	1.436
Calgary_24h_100y	18:30	1.429
Calgary_24h_100y	18:35	1.421
Calgary_24h_100y	18:40	1.414
Calgary_24h_100y	18:45	1.407
Calgary_24h_100y	18:50	1.399
Calgary_24h_100y	18:55	1.392
Calgary_24h_100y	19:00	1.385
Calgary_24h_100y	19:05	1.378
Calgary_24h_100y	19:10	1.372
Calgary_24h_100y	19:15	1.365
Calgary_24h_100y	19:20	1.358
Calgary_24h_100y	19:25	1.352
Calgary_24h_100y	19:30	1.345
Calgary_24h_100y	19:35	1.339
Calgary_24h_100y	19:40	1.332
Calgary_24h_100y	19:45	1.326
Calgary_24h_100y	19:50	1.32
Calgary_24h_100y	19:55	1.313
Calgary_24h_100y	20:00	1.307
Calgary_24h_100y	20:05	1.301
Calgary_24h_100y	20:10	1.295
Calgary_24h_100y	20:15	1.289
Calgary_24h_100y	20:20	1.284
Calgary_24h_100y	20:25	1.278
Calgary_24h_100y	20:30	1.272
Calgary_24h_100y	20:35	1.266
Calgary_24h_100y	20:40	1.261
Calgary_24h_100y	20:45	1.255
Calgary_24h_100y	20:50	1.25
Calgary_24h_100y	20:55	1.244
Calgary_24h_100y	21:00	1.239
Calgary_24h_100y	21:05	1.234
Calgary_24h_100y	21:10	1.229
Calgary_24h_100y	21:15	1.223
Calgary_24h_100y	21:20	1.218
Calgary_24h_100y	21:25	1.213
Calgary_24h_100y	21:30	1.208
Calgary_24h_100y	21:35	1.203
Calgary_24h_100y	21:40	1.198
Calgary_24h_100y	21:45	1.193
Calgary_24h_100y	21:50	1.188
Calgary_24h_100y	21:55	1.184
Calgary_24h_100y	22:00	1.179
Calgary_24h_100y	22:05	1.174
Calgary_24h_100y	22:10	1.17
Calgary_24h_100y	22:15	1.165
Calgary_24h_100y	22:20	1.16
Calgary_24h_100y	22:25	1.156
Calgary_24h_100y	22:30	1.151
Calgary_24h_100y	22:35	1.147
Calgary_24h_100y	22:40	1.143
Calgary_24h_100y	22:45	1.138
Calgary_24h_100y	22:50	1.134
Calgary_24h_100y	22:55	1.13

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Continuous Simulation

Calgary_24h_100y	23:00	1.125
Calgary_24h_100y	23:05	1.121
Calgary_24h_100y	23:10	1.117
Calgary_24h_100y	23:15	1.113
Calgary_24h_100y	23:20	1.109
Calgary_24h_100y	23:25	1.105
Calgary_24h_100y	23:30	1.101
Calgary_24h_100y	23:35	1.097
Calgary_24h_100y	23:40	1.093
Calgary_24h_100y	23:45	1.089
Calgary_24h_100y	23:50	1.085
Calgary_24h_100y	23:55	1.081
Calgary_24h_100y	24:00	1.077

IDE-Withdrawal FILE "C:\0-AM_Files\Projects\Kineticor Final\IDE
tool_PondWithdrawal.txt"

YYC_Precipitation FILE "C:\Users\Ajay.Muthukumar\Cima+\Z0026600-Kineticor-Hwy 566
North Calgary Site - Documents_Documents\300_CONC_DES\399_Stormwater\PCSWMM\Precip
1960-2014.dat"

YYC_Temp FILE "C:\Users\Ajay.Muthukumar\Cima+\Z0026600-Kineticor-Hwy 566
North Calgary Site -
Documents_Documents\300_CONC_DES\399_Stormwater\PCSWMM\CalgTempHrly1960_2014.dat"

[REPORT]

;;Reporting Options
INPUT YES
CONTROLS NO
SUBCATCHMENTS ALL
NODES ALL
LINKS ALL

[ADJUSTMENTS]

;;Parameter	Subcatchment	Monthly Adjustments								
CONDUCTIVITY			0.05	0.05	0.05	0.05	1.0	1.0	1.0	1.0
1.0	1.0	0.05	0.05							

[TAGS]

Subcatch	SC-1	Developed
Subcatch	SC-2	Developed
Subcatch	SC-3	Undeveloped
Subcatch	SC-POND	Pond

[MAP]

DIMENSIONS	11723.78165	5675083.43005	14425.01535	5677730.90895
UNITS	Meters			

[COORDINATES]

;;Node	X-Coord	Y-Coord
OF-IRRIGATION	14093.209	5676880.175
OF-N-1	14287.972	5677069.091
OF-N-2	14285.091	5677021.142
OF-S-1	14302.232	5675813.051
STG-DEVELOPMENT	13801.186	5676473.424
STG-POND	14164.545	5676480.257

[VERTICES]

;;Link	X-Coord	Y-Coord
;;-----	-----	-----

[POLYGONS]

Master Drainage Plan

Kineticor ASP

PCSWMM Input File

Continuous Simulation

```
;;Subcatchment X-Coord Y-Coord
;;-----
SC-1 13498.632 5677046.219
SC-1 12647.703 5677046.341
SC-1 12650.248 5676805.025
SC-1 11846.565 5676798.811
SC-1 11849.474 5675994.149
SC-1 12652.92 5675999.973
SC-1 12655.809 5675205.662
SC-1 14270.572 5675203.77
SC-1 14270.572 5676253.596
SC-1 14069.867 5676254.433
SC-1 14070.993 5676671.22
SC-1 13500.068 5676672.762
SC-1 13498.632 5677046.219
SC-2 12647.703 5677046.341
SC-2 12646.12 5677610.569
SC-2 13496.342 5677605.354
SC-2 13498.632 5677046.219
SC-2 12647.703 5677046.341
SC-3 13500.068 5676672.762
SC-3 13496.342 5677605.354
SC-3 14265.915 5677601.944
SC-3 14269.307 5676670.684
SC-3 13500.068 5676672.762
SC-POND 14071.7 5676671.927
SC-POND 14269.307 5676670.684
SC-POND 14270.572 5676253.596
SC-POND 14069.867 5676254.433
SC-POND 14071.7 5676671.927
```

```
;;Storage Node X-Coord Y-Coord
;;-----
```

[SYMBOLS]

```
;;Gage X-Coord Y-Coord
;;-----
```

EPA STORM WATER MANAGEMENT MODEL - VERSION 5.2 (Build 5.2.4)

Element Count

Number of rain gages 3
Number of subcatchments ... 4
Number of nodes 6
Number of links 4
Number of pollutants 0
Number of land uses 0

Raingage Summary

Name	Data Source	Data Type	Recording Interval
Calgary_24h_100y	Calgary_24h_100y	INTENSITY	5 min.
Calgary_Cont_1960-2014	YYC_Precipitation	INTENSITY	60 min.
IDE_SC-3_Rainfall	C:\0-AM_Files\Projects\Kineticor Final\IDE tool_SC-3_irr.dat		

Subcatchment Summary

Name	Area	Width	%Imperv	%Slope	Rain Gage
Outlet					
SC-1	325.50	32550.00	75.50	3.0000	Calgary_Cont_1960-2014
STG-DEVELOPMENT					
SC-2	47.80	4780.00	75.50	3.0000	Calgary_Cont_1960-2014
STG-DEVELOPMENT					
SC-3	71.70	1434.00	0.00	4.0000	Calgary_Cont_1960-2014
OF-N-1					
SC-POND	8.30	1660.00	86.75	20.0000	Calgary_Cont_1960-2014
STG-POND					

Node Summary

Name	Type	Invert Elev.	Max. Depth	Ponded Area	External Inflow
OF-IRRIGATION	OUTFALL	1025.00	0.00	0.0	
OF-N-1	OUTFALL	1018.00	0.00	0.0	
OF-N-2	OUTFALL	1022.50	0.00	0.0	
OF-S-1	OUTFALL	1033.00	0.00	0.0	
STG-DEVELOPMENT	STORAGE	1025.00	0.30	0.0	
STG-POND	STORAGE	1019.50	5.50	0.0	

Link Summary

Master Drainage Plan

Kineticor ASP

PCSWMM Output File

Continuous Simulation

Name	From Node	To Node	Type	Length	%Slope
Roughness					
P-IRRIGATION	STG-POND	OF-IRRIGATION	TYPE2 PUMP		
P-POND-OUT	STG-POND	OF-S-1	TYPE2 PUMP		
OR-POND-OUT	STG-POND	OF-N-2	ORIFICE		
OL-POND-IN	STG-DEVELOPMENT	STG-POND	OUTLET		

Cross Section Summary

		Full	Full	Hyd.	Max.	No. of
Full						
Conduit	Shape	Depth	Area	Rad.	Width	Barrels
Flow						

Analysis Options

Flow Units CMS

Process Models:

Rainfall/Runoff YES

RDII NO

Snowmelt YES

Groundwater NO

Flow Routing YES

Ponding Allowed NO

Water Quality NO

Infiltration Method HORTON

Flow Routing Method DYNWAVE

Surcharge Method EXTRAN

Starting Date 01/01/1960 00:00:00

Ending Date 12/31/2014 23:00:00

Antecedent Dry Days 0.0

Report Time Step 01:00:00

Wet Time Step 00:15:00

Dry Time Step 00:15:00

Routing Time Step 60.00 sec

Variable Time Step YES

Maximum Trials 8

Number of Threads 1

Head Tolerance 0.001500 m

Rainfall File Summary

Station	First	Last	Recording	Periods	Periods	Periods
ID	Date	Date	Frequency	w/Precip	Missing	Malfunc.
1001093	01/02/1960	12/29/2014	60 min	45403	0	0

Runoff Quantity Continuity

Volume

hectare-m

Depth

mm

Master Drainage Plan

Kineticor ASP

PCSWMM Output File

Continuous Simulation

Initial Snow Cover	0.000	0.000
Total Precipitation	10375.130	22888.000
Evaporation Loss	2399.994	5294.494
Infiltration Loss	4688.212	10342.406
Surface Runoff	3372.369	7439.597
Snow Removed	0.000	0.000
Final Snow Cover	1.179	2.600
Final Storage	0.163	0.360
Continuity Error (%)	-0.836	

	Volume hectare-m	Volume 10^6 ltr
*****	-----	-----
Flow Routing Continuity	0.000	0.000
Dry Weather Inflow	3372.369	33724.045
Wet Weather Inflow	0.000	0.000
Groundwater Inflow	0.000	0.000
RDII Inflow	0.000	0.000
External Inflow	0.000	0.000
External Outflow	3084.452	30844.839
Flooding Loss	0.000	0.000
Evaporation Loss	290.505	2905.083
Exfiltration Loss	0.000	0.000
Initial Stored Volume	13.708	137.083
Final Stored Volume	18.111	181.110
Continuity Error (%)	-0.206	

Time-Step Critical Elements

None

Highest Flow Instability Indexes

All links are stable.

Most Frequent Nonconverging Nodes

Convergence obtained at all time steps.

Routing Time Step Summary

Minimum Time Step	:	59.50 sec
Average Time Step	:	60.00 sec
Maximum Time Step	:	60.00 sec
% of Time in Steady State	:	0.00
Average Iterations per Step	:	2.00
% of Steps Not Converging	:	0.00
Time Step Frequencies	:	
60.000 - 23.031 sec	:	100.00 %
23.031 - 8.841 sec	:	0.00 %
8.841 - 3.393 sec	:	0.00 %
3.393 - 1.303 sec	:	0.00 %
1.303 - 0.500 sec	:	0.00 %

Subcatchment Runoff Summary

Perv	Total	Total	Total	Total	Total	Total	Imperv
Runoff	Runoff	Precip	Peak	Runoff	Evap	Infil	Runoff
Subcatchment	Runoff	Runoff	Runoff	Coeff	mm	mm	mm
mm	mm	10^6 ltr	mm	mm	mm	mm	mm
			CMS				
SC-1		22888.00		0.00	6321.92	8193.42	11258.60
714.20	8595.22	27977.60	46.23	0.376			
SC-2		22888.00		0.00	6321.92	8193.42	11258.60
714.20	8595.22	4108.54	6.79	0.376			
SC-3		22888.00		0.00	289.92	22388.41	0.00
217.25	217.25	155.77	2.18	0.009			
SC-POND		22888.00		0.00	2317.49	2935.16	17786.29
68.87	17855.16	1481.99	1.21	0.780			

Node Depth Summary

Node	Type	Average Depth Meters	Maximum Depth Meters	Maximum HGL Meters	Time of Max Occurrence days hr:min	Reported Max Depth Meters
OF-IRRIGATION	OUTFALL	0.00	0.00	1025.00	0 00:00	0.00
OF-N-1	OUTFALL	0.00	0.00	1018.00	0 00:00	0.00
OF-N-2	OUTFALL	0.00	0.00	1022.50	0 00:00	0.00
OF-S-1	OUTFALL	0.00	0.00	1033.00	0 00:00	0.00
STG-DEVELOPMENT	STORAGE	0.00	0.27	1025.27	17322 20:10	0.25
STG-POND	STORAGE	2.72	5.42	1024.92	3816 16:06	5.42

Node Inflow Summary

Total	Flow		Maximum	Maximum		Lateral
Inflow	Balance		Lateral	Total	Time of Max	Inflow
Volume	Error		Inflow	Inflow	Occurrence	Volume
Node	Type		CMS	CMS	days hr:min	10^6 ltr
ltr	Percent					10^6
OF-IRRIGATION	OUTFALL		0.000	0.245	121 00:01	0
2.25e+04	0.000					
OF-N-1	OUTFALL		2.177	2.177	17322 20:00	156
156	0.000					

Master Drainage Plan

Kineticor ASP

PCSWMM Output File

Continuous Simulation

OF-N-2		OUTFALL	0.000	1.504	3816	16:07	0
1.4e+03	0.000						
OF-S-1		OUTFALL	0.000	2.200	3816	15:07	0
6.82e+03	0.000						
STG-DEVELOPMENT		STORAGE	53.020	53.020	17322	20:00	3.21e+04
3.21e+04	-0.004						
STG-POND		STORAGE	1.205	34.805	17322	20:00	1.48e+03
3.31e+04	-0.207						

Node Surcharge Summary

No nodes were surcharged.

Node Flooding Summary

No nodes were flooded.

Storage Volume Summary

Max	Maximum	Average	Avg	Evap	Exfil	Maximum	Max	Time of
Occurrence	Outflow	Volume	Pcnt	Pcnt	Pcnt	Volume	Pcnt	days
Storage Unit	Storage Unit	1000 m³	Full	Loss	Loss	1000 m³	Full	
hr:min	CMS							
STG-DEVELOPMENT		0.030	0.0	1.8	0.0	75.455	89.8	17322
20:10	33.615							
STG-POND		136.392	43.4	7.0	0.0	308.059	98.1	3816
16:06	3.708							

Outfall Loading Summary

Outfall Node	Flow Freq Pcnt	Avg Flow CMS	Max Flow CMS	Total Volume 10^6 ltr
OF-IRRIGATION	5.29	0.245	0.245	22473.890
OF-N-1	0.13	0.068	2.177	155.768
OF-N-2	0.59	0.138	1.504	1399.679
OF-S-1	0.68	0.580	2.200	6815.362
System	1.67	1.030	5.153	30844.698

Master Drainage Plan

Kineticor ASP

PCSWMM Output File

Continuous Simulation

Link Flow Summary

Link	Type	Maximum Flow CMS	Time of Max Occurrence days hr:min		Maximum Veloc m/sec	Max/ Full Flow	Max/ Full Depth
P-IRRIGATION	PUMP	0.245	121	00:01		1.00	
P-POND-OUT	PUMP	2.200	3816	15:07		1.00	
OR-POND-OUT	ORIFICE	1.504	3816	16:07			1.00
OL-POND-IN	DUMMY	33.600	17322	19:50			

Flow Classification Summary

Conduit	Adjusted /Actual Length	Fraction of Time in Flow Class								
		Up		Down	Sub	Sup	Up	Down	Norm	Inlet
		Dry	Dry	Dry	Crit	Crit	Crit	Crit	Ltd	Ctrl

Conduit Surcharge Summary

No conduits were surcharged.

Pumping Summary

Power Usage Kw-hr	% Time Off		Percent Utilized	Number of Start-Ups	Min	Avg	Max	Total
	Pump	Curve			Flow	Flow	Flow	Volume
	Low	High			CMS	CMS	CMS	10^6 ltr
P-IRRIGATION			5.29	6419	0.00	0.25	0.24	22473.890
173981.62	0.0	0.0						
P-POND-OUT			0.68	25174	0.00	0.58	2.20	6815.362
182421.00	0.0	0.0						

Analysis begun on: Mon Jul 21 21:08:30 2025

Analysis ended on: Mon Jul 21 21:08:55 2025

Total elapsed time: 00:00:25